KLEIDERMACHER RESIDENCE

385 LOMA MEDIA ROAD SANTA BARBARA, CA

INTERIOR AND EXTERIOR ALTERATIONS TO EXISTING SINGLE FAMILY RESIDENCE

GENERAL REQUIREMENTS

All work shall be performed in a professional workmanlike manner by a licensed contractor and shall comply with the following:

- 1. These General notes unless otherwise noted on plans or specifications.
- 2. This project shall comply with:
 - 2016 California Building Code (Title 24) 2016 CRC (or current edition) 2016 CMC (or current edition)
 - 2016 CPC (or current edition) 2016 CEC (or current edition) California Efficiency Standards
 - California Green Building Standards 2016 (Cal Green) or current The New Zoning Ordinance TITLE 30 City of Santa Barbara ORDINANCE #5780
- 3. All applicable Local, State and Federal Codes, Ordinances, Laws, Regulations and Protective Covenants governing the site of work.
- 4. Standard Specifications of ASTM
- 5. In case of conflicts, the more stringent requirements shall govern.
- 6. "Or Equal": the Contractor shall submit for the Designer's and Builder's approval all materials or equipment which is considered "or equal" to that specified.

On Site Verification of all dimensions and conditions shall be the responsibility of the Contractor and Sub-contractors. Noted dimensions to take precedence over scale. Each Contractor and Subcontractor shall report to designer and Project Superintendent all Conditions which prevent the proper execution of their work. Client's Architect and Project Superintendent to be notified immediately by Contractor and Sub-Contractor should any discrepancy, error, omission, addition, or other question arise pertaining to the working drawings and/or specifications. The Contractor shall be held responsible for the results of any errors, discrepancies, or omissions which the Contractor failed to notify the Architect of before construction and/or fabrication of the work. Sub- contractor shall: Insure that all work is done in a professional workmanlike

manner by skilled mechanics and shall replace any materials or items damaged by Sub-contractor performance. Sub-contractors and suppliers are hereby notified that they are to confer and cooperate fully with each other during the course of construction to determine the exact extent and overlap of each other's work and to successfully complete the execution of the work. All Sub-contractor workmanship will be of quality to pass inspections by local authorities, lending institutions, Architect or Builder. Any one or all of the above mentioned inspectors may inspect workmanship at any time, and any corrections needed to enhance the quality of building will be done immediately. The Contractor shall, at all times, provide protection against weather, rain, wind, storms, frost, heat or breakage so as to maintain all work, materials, apparatus, and fixtures free from injury or damage. The contractor shall at all times keep the premises free from all accumulations of waste material or rubbish caused by his employees, and at the completion of the work shall remove all rubbish, debris, equipment and surplus materials belonging to him from in and about the building and leave work site "Broom Clean". At the end of the day's work, all work likely to be damaged shall be covered. Any work damaged due to failure of protection as defined above shall be removed and replaced with new work at the Contractor's expense. Each Sub-contractor, unless specifically exempted by the terms of his sub-contract agreement, shall be responsible for cleaning up and removing from the job site all trash and debris not left by other Sub-contractors. Builder will determine how soon after Sub-contractor completes each phase of his work that trash and debris will be removed from the

Workmanship: Compliance with drawings and/or specifications with regard to materials and methods of assembly will not, in itself, assure acceptance of the work. Of equal importance is good workmanship, the lack of which will be sufficient cause to refuse acceptance of the work.

BEST MANAGEMENT PRACTICES



Building & Safety Division **Best Management Practices**



Best Management Practices for Construction Activities:

- □ Eroded sediments and other pollutants must be retained on site and may not be transported from the site via sheet flow, swales, area drains, natural drainage courses or wind. Stockpiles of earth and other construction related materials must be protected from being transported.
- from the site by the forces of wind or water. □ Fuels, oils, solvents and other toxic materials must be stored in accordance with their listing and are not to contaminate the soil and surface waters. All approved storage containers are to be protected from the weather. Spills may not be washed into the drainage system.
- DExcess or waste concrete may not be washed into the public way or any other drainage system Provisions must be made to retain concrete wastes on site until they can be disposed of as a solid waste. □ Trash and construction related solid wastes must be deposited into a covered receptacle to prevent
- contamination of rainwater and dispersal by wind. Description Sediments and other material may not be traced from the site by vehicle traffic. The construction entrance roadways must be stabilized so as to inhibit sediments from being deposited into the public way Accidental depositions must be swept up immediately and may not be washed down by rain or other
- Any slopes with disturbed soils or demanded of vegetation must be stabilized so as to inhibit erosion by

SPECIAL INSPECTION

REFER TO SHEET S 1.0 FOR SPECIAL INSPECTION NOTES

ABBREVIATIONS

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& L @ CL f (E) (N) # A.B. US. A.D. ADJ. AGUM. APPROX. ASPH. BLDG. BM. B.O. C.B. C.J. C.J. C.J. C.CONN. C.CONN.T. C.ONSTR. CONT. CORR. C.M.U. CTSK.	And Angle At Center-line Diameter Existing New Pound or Number Anchor Bolt Acoustical Area Drain Adjustable Aggregate Aluminum Approximate Architectural Asphalt Board Building Blocking Beam Bottom of Cabinet Catch Basin Cement Ceramic Cast Iron Control Joint Ceiling Calking Closet Clear Cleanout Column Concrete Connection Construction Control Jont Control Joint Ceiling Calking Calking Closet Clear Cleanout Column Concrete Connection Construction Contrinuous Corrugated Concrete Masonry Unit Countersunk	FDN. FIN. FL. FLASH. FLOUR. F.O.C. F.O.M. F.O.S. FT. FTG. FURR. FAALV. G.F.I. GL.B. GND. GR. GYP. H.D. HDR. HORIZ. HR. HT. I.D. INSUL. INT. JT. KIT. LAM. LAV. LIN. LT.	Foundation Finish Floor Flashing Fluorescent Face of Concrete Face of Finish Face of Masonry Face of Stud Full size Foot or Feet Footing Furring Gauge Galvanized Ground Fault Interrupter Glass Glued Laminated Beam Ground Grade Gypsum Hose Bid Hold Down Header Hardware Horizontal Hour Height Inside Diameter Insulation Interior Joint Kitchen Laminate Lavatory Linen Light	PL. P.LAM. PLAS. PLYWD. PR. PR. PR. P.T. Q.T. R. RAD. REF. REFR. REINF. REO'D. RESIL. R.O. R.W.L. S.CHED. SECT. S.F. SHWR. SHT. SIM. SPEC. STD. STRUCT. SUSP. S.W. SYM. TRD. T.C.	Plate PlasticLaminate Plaster Plywood Pair Pre-cast Pressure-treated Quarry Tile Riser Radius Roof Drain Reference Refrigerator Register Reinforced Required Resilient Room Rough Opening Redwood Rain Water Leader South Schedule Section Square Feet Shelf Shelf and Pole Shower Sheet Similar Specification Square Standard Steel Storage Structural Suspended Shearwall Symmetrical Tread Towel Bar Top of Curb
CNTR. CTR. d. DBL. DEPT. DET'L DIA. DIM. D.J.	Counter Center Penny Double Department Detail Diamete Dimension	L.V.L. MAX. M.B. M.C. MECH. MTL.	Laminated Veneer Maximum Machine Bolt Medicine Cabinet Mechanical Metal	TEL. T & G THK. T.O. T.P. T.P.D.	Telephone Tounge and Groove Thick Top of Top of Pavement Toilet Paper Dispenser Television
DN. DR. DWR. DS. DWG.	Doweled joint Down Door Drawer Downspout Drawing	MFR. MIN. MIR. MISC. M.O.	Manufacturer Minimum Mirror Miscellaneous Masonry Opening North	T.S. TYP. UBC UNF. U.N.O.	Tube Steel Typical Uniform Building Code Unfinished Unless Noted Otherwise
E. EA. E.J. EL. ELEC. ELEV. ENCL. EQ. EQPT. EXP. EXT.	East Each Expansion Joint Elevation Electrical Elevator Enclosure Equal Equipment Expansion Exterior	N.I.C. NO./# NOM. N.T.S. O.A. O.C. O.D. OFF. OPNG. OPP.	Not in Contract Number Nominal Not to Scale Overall On Center Outside Diameter Office Opening Opposite	VERT. W. W/ W.C. WD. W/O WP. W.R. WSCT. WT.	Vertical West With Water Closet Wood Without Waterproof Water Resistant Wainscot Weight

PROFESSIONALS

EXISTING & PROPOSED SQUARE FOOTAGE

OCUITECT.	
RCHITECT:	

SHERRY AND ASSOCIATES, ARCHITECTS 535 SANTA BARBARA STREET SANTA BARBARA, CA 93101 PH: (805) 963-0986

STRUCTURAL ENGINEER:

226 EAST CANON PERDIDO STREET #A SANTA BARBARA, CA 93101

ENERGY CONSULTANT:

8020 DOTTIE CT. BAKERSFIELD, CA 93308 PH: (661) 587-3125

FAX: (805) 963-0178

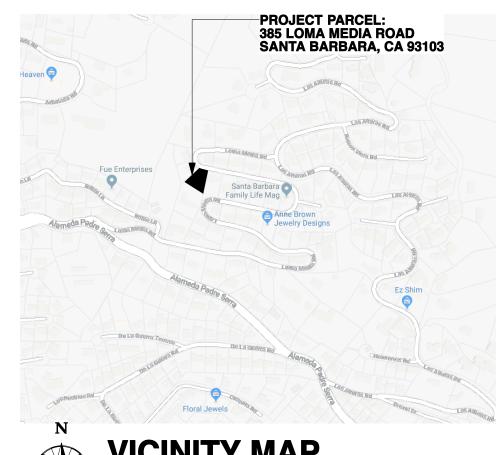
PH: (805) 568-0880

SQUARE FOOTAGE BREAKDOWN

(NO CHANGES PROPOSED):	<u>NET</u>	GROSS
(E) LOWER LEVEL FLOOR PLAN:	± 1,020 SF	± 1,087 SF
(E) UPPER LEVEL FLOOR PLAN:	± 1,365 SF	± 1,443 SF
TOTAL PROPOSED RESIDENCE (NO CHANGES):	± 2,385 SF	± 2,530 SF
(E) 2 CAR GARAGE:	± 402 SF	± 419 SF
TOTAL PROPOSED RESIDENCE + GARAGE (NO CHANGES):	± 2,787 SF	± 2,949 SF
(E) LOWER LEVEL DECK:		± 729 SF
(E) UPPER LEVEL DECK:		± 621 SF
AREAS OF ALTERATION:		
LOWER LEVEL REMODEL:		± 913 SF
UPPER LEVEL REMODEL:		± 703 SF
(N) TRELLIS AT REAR DECK:		± 613 SF
IMPERVIOUS AREAS:		
(E) IMPERVIOUS SURFACES (NO CHANGES):		± 3,500 SF
(N) IMPERVIOUS SURFACES:		± 0 SF
REPLACED IMPERVIOUS SURFACE (RE ROOFING):		± 1,862 SF*
REMOVED IMPERVIOUS SURFACES:		± 0 SF
*NOTE: PER APPENDIX J OF THE STORM WATER BMP GU		

PROJECTS INVOLVING NO INCREASE IN ROOF SURFACE AREA ARE EXEMPT FROM TIER 3

REQUIREMENTS. TREAT AS TIER 2 IF ROOF WORK IS OVER 500 SF.





PROJECT STATIS	STICS
OWNER:	DAVID & TAMARA KLEIDERMACHER 385 LOMA MEDIA ROAD SANTA BARBARA, CA 93103
PROJECT ADDRESS:	385 LOMA MEDIA ROAD SANTA BARBARA, CA 93103
APN:	019-261-024
LAND USE ZONE:	RS-15 (TITLE 30)
GENERAL PLAN:	RIVIERA: RIVIERA
PROPERTY USE:	SINGLE FAMILY RESIDENTIAL
CONSTRUCTION TYPE:	V-B
OCCUPANCY TYPE:	R-3 (NO CHANGES)
AVERAGE SLOPE:	± 40.31%
HIGH FIRE:	YES
HILLSIDE DESIGN DISTRICT:	YES
PARKING:	2 CAR GARAGE (NO CHANGES)
LOT SIZE:	± 11,001 SF (± 0.25 ACRES)

PROJECT DESCRIPTION

NEW OR REPLACED IMPERVIOUS

INTERIOR AND EXTERIOR ALTERATIONS TO EXISTING SINGLE FAMILY RESIDENCE
EXTERIOR

(N) GARAGE DOOR W/ LITES TO REPLACE (E)
(N) ENTRY DOOR WITH SIDELIGHTS TO REPLACE (E) ENTRY DOOR
(N) EYEBROW TRELLISES ABOVE GARAGE DOOR & ENTRY DOOR
(N) EXTERIOR LIGHTS
ŘÉ ROOFING, REUSE (E) CLAY TILES
 (E) GUARDRAILS TO BE REPLACED BY (N) WROUGHT IRON GUARDRAILS
(É) GUARDRAILS TO BE REPLACED BY (N) SS CABLE GUARDRAILS AT RÉAR DECKS)
(N) TRELLIS TO REPLACE (E) AT REAR UPPER DECK
(N) WINDOWS & DOORS PÈŔ PLANS

REPLACE (E) WINDOWS WITH (N) PER PLANS REMOVE (E) WROUGHT IRON GRATES AT (E) WINDOWS PER PLANS (N) SMOOTH PLASTER TO REPLACE (E) STUCCO FINISH

INTERIOR REMODEL PER PLANS REMOVE (E) FIREPLACE & CHIMNEY (N) DIRECT VENT FIREPLACE RÉLOCATE KITCHEN AT UPPER LEVEL RELOCATE MASTER BEDROOM TO LOWER LEVEL (N) W/C & (N) MUD ROOM AT UPPER LEVEL (N) LAUNDRY ROOM AT LOWER LEVEL (N) MASTER BATHROOM AT LOWER LEVEL ÀLTERED (E) BEDROOM & (E) BATHROOM AT LOWER LEVEL

ABATEMENT OF VIOLATIONS IN ZIR2018-00192

) CONNECT DOWNSPOUTS TO RAIN BARRELS

SHEET INDEX

1.0	TITLE SHEET / SITE PLAN
0.1 0.2 1.1 1.2 2.1 2.2 2.3 2.4 3.1 4.1 5.1 5.2 5.3	LOWER LEVEL FLOOR PLAN - EXISTING/DEMO UPPER LEVEL FLOOR PLAN - EXISTING/DEMO LOWER LEVEL FLOOR PLAN - PROPOSED UPPER LEVEL FLOOR PLAN - PROPOSED EXTERIOR ELEVATIONS - EXISTING EXTERIOR ELEVATIONS - EXISTING EXTERIOR ELEVATIONS - PROPOSED EXTERIOR ELEVATIONS - PROPOSED EXTERIOR ELEVATIONS - PROPOSED SECTION - PROPOSED DOOR / WINDOW SCHEDULES ARCHITECTURAL DETAILS ARCHITECTURAL DETAILS ARCHITECTURAL DETAILS
1.0 1.1 1.2 2.1 2.2 2.3 2.4	GENERAL NOTES LOWER LEVEL FRAMING PLAN - PROPOSED UPPER LEVEL FRAMING PLAN - PROPOSED TYPICAL DETAILS TYPICAL DETAILS STRUCTURAL DETAILS STRUCTURAL DETAILS

OPEN YARD REQUIREMENT (E) REAR DECK TO REMAIN, ± 621 SF (N) TRELLIS TO ŘÉPLACE (E)-(E) RESIDENCE APŃ: 019-261-024 ADJACENT PROPERTY -RE ROOF, REUSE (E) CLAY TILES. APN: 019-261-006 ± 1,862 SF (NO INCRÉASE IN ROOF SURFACE ÀREA) (E) 2 CAR GARAGE `B'ACK (NO CHANGES)--(N) A/C CONDENSER LOCATION, ± 11'-6" TO PROPERTY LINE, **REFER TO DETAIL 2/A 5.3** (E) TILED ENTRY COURTYARD TÓ REMAIN, ± 344 SF -ADJACENT PROPERTY APN: 019-271-009 (N) GARAGE EYEBROW TRELLIS-(N) 42" HIGH WROUGHT IRON -(N) EYEBROW TRELLIS OVER FRONT ENTRY GÚARDRAIL TO REPLACE (E)--(N) 42" HIGH WROUGHT IRON GÚARDRAIL TO REPLACE (E) PORTION OF GUARDRAIL FROM DASHED LINE TO PUBLIC RIGHT OF WAY TO BE MEASURED FROM ADJACENT PUBLIC RIGHT OF WAY, SEE NOTE (E) 5' HIGH HEDGE TO REMAIN— 42.14' S78°32'30"E (E) CONCRETE DRIVEWAY TO REMAIN, ± 673 SF 10.50' R=65.0' (E) TERRACED LANDSCAPE AREA--(N) PILASTERS & WROUGHT IRON GATE - 42" MAX (E) ± 36" HIGH WROUGHT IRON FENCE TO BE REPLACED WITH (N) 42" HIGH WROUGHT IRON GUARDRÁIL-



21 SHEETS TOTAL

LOMA MEDIA ROAD SITE PLAN - EXISTING & PROPOSED

LEGEND

■ WATER RUN OFF FLOW

TIER 2 SWMP COMPLIANCE NOTES:

1) CONNECT DOWNSPOUTS TO RAIN BARRELS

NOTES:

COMMUNITY DEVELOPMENT DIRECTOR, PURSUANT TO SBMC 30.140.110.E.2.C, APPROVED A REQUEST TO ALLOW A PORTION OF THE 42" HIGH WROUGHT IRON GUARDRAIL TO BE MEASURED FROM THE ELEVATION OF THE ADJACENT PUBLIC RIGHT-OF-WAY (LOMA MEDIA ROAD). REFER TO SITE PLAN FOR DELINEATION OF WROUGHT IRON GUARDRAIL SUBJECT TO THIS ÁPPROVAL.(PLN2018-00510)

ALL WORK IN THE RIGHT-OF-WAY REQUIRES A SEPARATE PERMIT FROM PUBLIC WORKS. CONFIRM WITH SURVEY PRIOR TO CONSTRUCTION

DATE DESCRIPTION 09/25/18 SFDB SUBMITTAL 10/08/18 | SFDB RE-SUBMITTAL

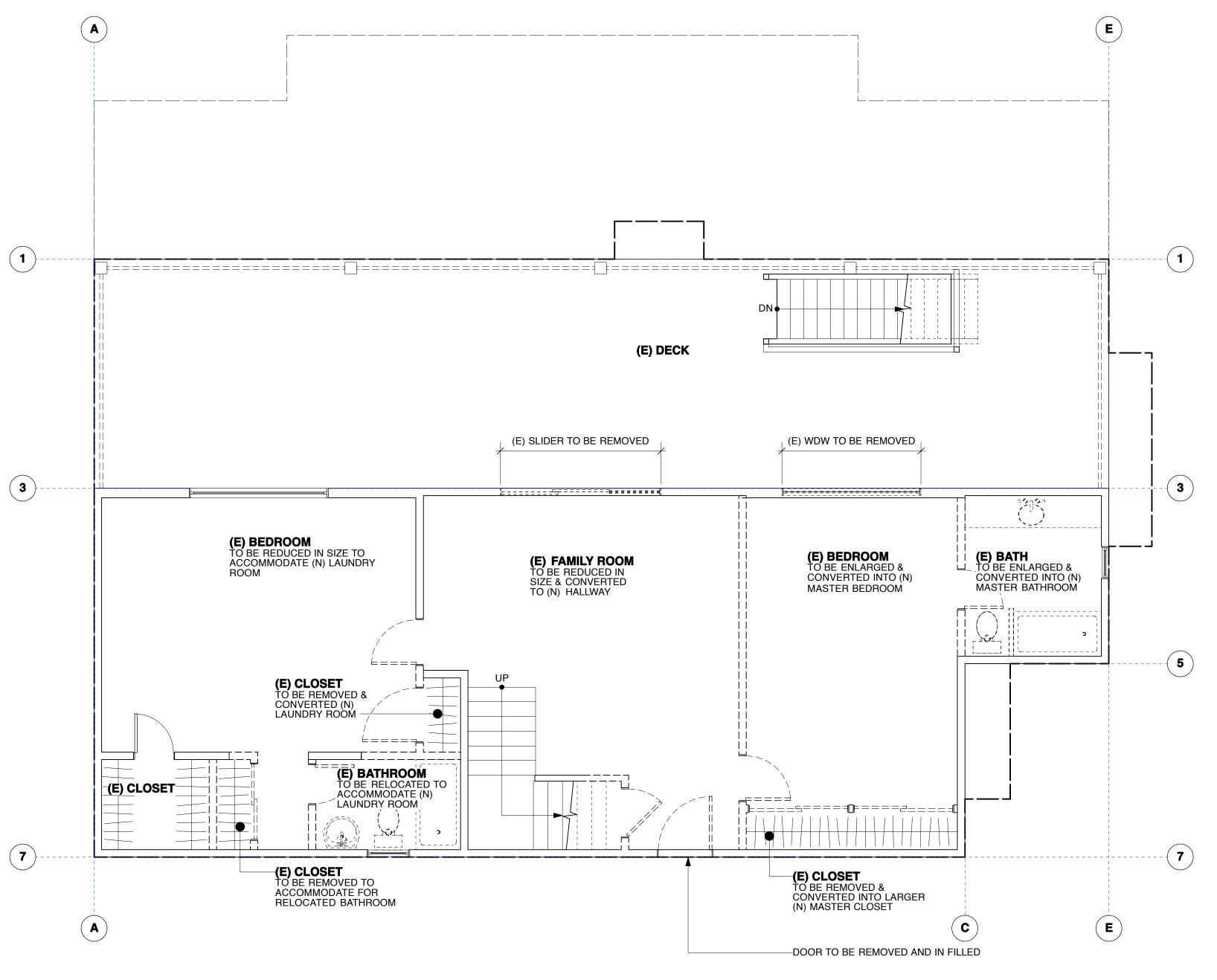
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01/14/19 SFDB RE-SUBMITTAL 02/21/19 BLDG DEPT SUBMITTAL

04/17/19 BLDG DEPT RE-SUBMITTAL 10/31/19 SFDB REVISION #1 SUBMITTAL

EID

TITLE SHEET / SITE PLAN





DEMOLITION NOTES

ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCY REGARDING NON-BEARING / BEARING WALLS. NO BEARING WALLS SHALL BE REMOVED WITHOUT ARCHITECT / STRUCTURAL ENGINEER REVIEW AND APPROVAL

DEMOLITION SHALL BE LIMITED TO THE SPECIFIC DAYS AND HOURS OF CONSTRUCTION AS SET FORTH BY THE DEPARTMENT OF BUILDING AND SAFETY, CITY OF SANTA BARBARA

NO DEMOLITION WORK SHALL BE COMMENCED PRIOR TO OWNER AUTHORIZATION OF THESE PLANS, OWNER VERIFICATION OF EXTENT OF DEMOLITION, OR CITY OF SANTA BARBARA BUILDING DEPARTMENT APPROVAL.

STRUCTURAL ITEMS SHOWN "TO BE REMOVED" SHALL BE VERIFIED WITH ENGINEER FOR APPROPRIATE SHORING (IF APPLICABLE).

CONTRACTOR SHALL VERIFY ALL EXISTING STRUCTURAL CONDITIONS PRIOR TO DEMOLITION

ARCHITECT IS NOT RESPONSIBLE FOR REMOVAL OR ABATEMENT OF LEAD PAINT OR ASBESTOS. CONTRACTOR SHALL PROVIDE REMOVAL AND ABATEMENT PER LOCAL JURISDICTION REQUIREMENTS

EXTREME CARE SHALL BE TAKEN REGARDING THE REMOVAL OF:
FINISHES
STRUCTURAL ELEMENTS
MECHANICAL SYSTEMS
PLUMBING SYSTEMS

ELECTRICAL SYSTEMS

EXTREME CARE SHALL BE TAKEN TO VERIFY ALL FIELD CONDITIONS WITH ARCHITECT AND ENGINEER PRIOR TO DEMOLITION

ANY REMOVAL OF EXISTING FOUNDATION SHALL BE DETERMINED BY CITY OF SANTA BARBARA AND BASED ON CITY REQUIREMENTS. CITY BUILDING INSPECTOR SHALL DETERMINE EXTENT OF REMOVAL ON SITE AT THE TIME OF (BUT PRIOR TO) ANY DEMOLITION.

WALL LEGEND

(E) WALL TO REMAIN

(E) WALL TO REMOVED

DATE DESCRIPTION

09/25/18 SFDB SUBMITTAL

10/08/18 SFDB RE-SUBMITTAL

01/14/19 SFDB RE-SUBMITTAL

02/21/19 BLDG DEPT SUBMITTAL

04/17/19 BLDG DEPT RE-SUBMITTAL

10/31/19 SFDB REVISION #1 SUBMITTAL

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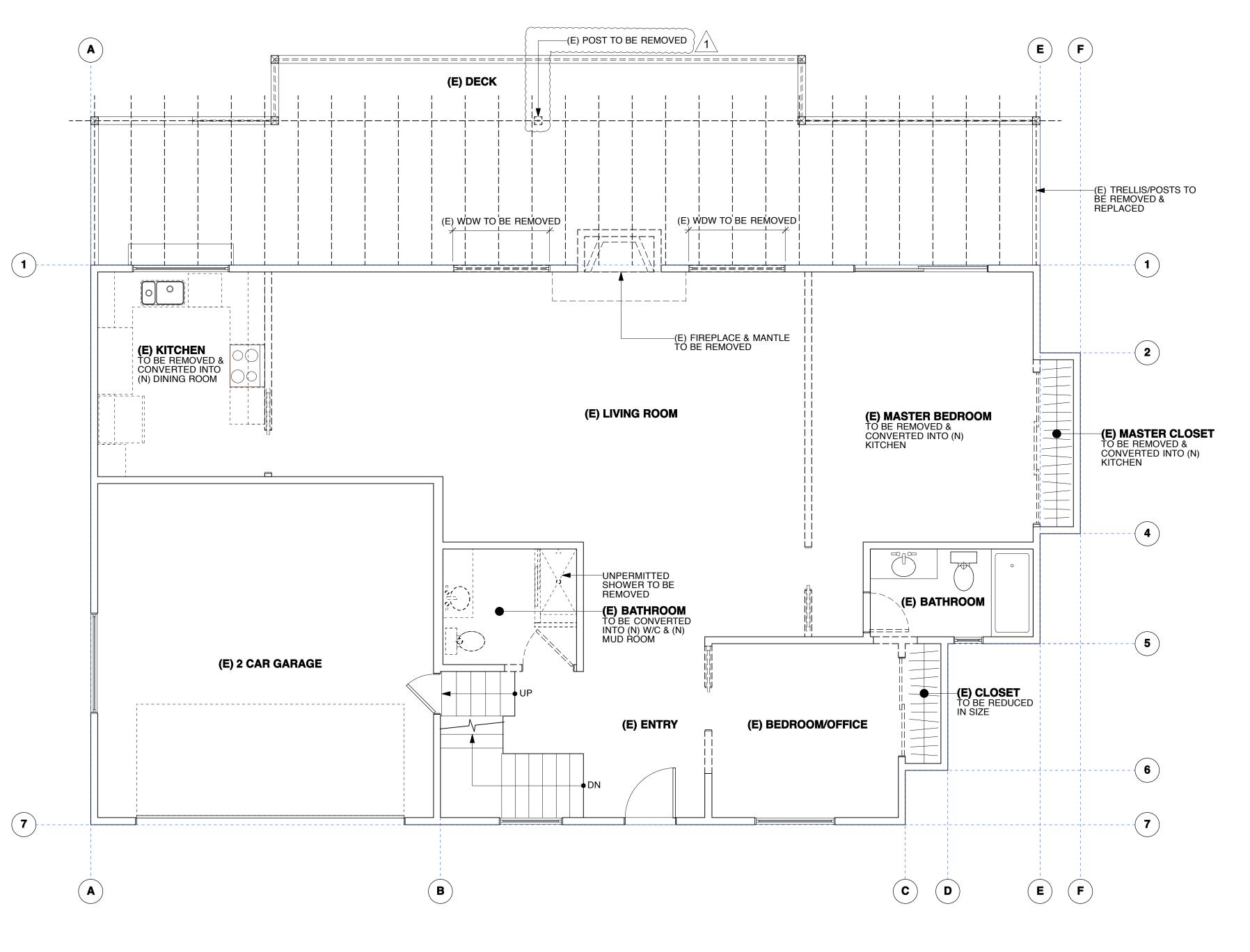
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AND EXTERIOR ALTERATIONS SINGLE FAMILY RESIDENCE

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LOWER LEVEL FLOOR PLAN -EXISTING & DEMO





DEMOLITION NOTES

ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCY REGARDING NON-BEARING / BEARING WALLS. NO BEARING WALLS SHALL BE REMOVED WITHOUT ARCHITECT / STRUCTURAL ENGINEER REVIEW AND APPROVAL

DEMOLITION SHALL BE LIMITED TO THE SPECIFIC DAYS AND HOURS OF CONSTRUCTION AS SET FORTH BY THE DEPARTMENT OF BUILDING AND SAFETY, CITY OF SANTA BARBARA

NO DEMOLITION WORK SHALL BE COMMENCED PRIOR TO OWNER

NO DEMOLITION WORK SHALL BE COMMENCED PRIOR TO OWNER AUTHORIZATION OF THESE PLANS, OWNER VERIFICATION OF EXTENT OF DEMOLITION, OR CITY OF SANTA BARBARA BUILDING DEPARTMENT APPROVAL.

STRUCTURAL ITEMS SHOWN "TO BE REMOVED" SHALL BE VERIFIED WITH ENGINEER FOR APPROPRIATE SHORING (IF APPLICABLE).

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MECHANICAL SYSTEMS

MECHANICAL SYSTEMS
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WALL LEGEND

(E) WALL TO REMOVED

DATE DESCRIPTION

09/25/18 SFDB SUBMITTAL

10/08/18 SFDB RE-SUBMITTAL

01/14/19 SFDB RE-SUBMITTAL

02/21/19 BLDG DEPT SUBMITTAL

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ALTERATIONS | RESIDENCE

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INTERIOR EXISTING

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04/17/19 BLDG DEPT RE-SUBMITTAL

10/31/19 SFDB REVISION #1 SUBMITTAL

UPPER LEVEL FLOOR PLAN -EXISTING & DEMO

FIRE NOTES:

ALL ELEMENTS OF NEW AND RE-BUILT CONSTRUCTION MUST COMPLY WITH REQUIREMENTS SET FORTH IN SECTION R337 OF THE 2016 CRC AND S.B. ORDINANCE NO. 5780.

1. ALL NEW ROOF UNDERLAYMENT TO BE CLASS A - TARCO LEAKBARRIER EASYSTICK PLUS AND EASYBASE MEMBRANE (ESR-3297) OR EQUIVALENT.

2. ALL NEW EXTERIOR WALLS TO BE 1-HR RATED. - REFER TO DETAIL 1/A 5.1.

3. ALL NEW ROOF SHETHING ON THE UNDERSIDE OF UNENCLOSED ROOF EAVES TO CONSIST OF 5/8" TYPE 'X' GYPSUM BOARD. - CERTAIN TEED CORP. 5/8" TYPE 'X' EXTERIOR SOFFIT BOARD OR EQUIVALENT.

4. ALL EXPOSED WOOD TO BE FIRE-RETARDANT-TREATED WOOD*. - HOOVER TREATED LUMBER W/ EXTERIOR FIRE X OR EQUIVALENT.

*FIRE-RETARDANT-TREATED WOOD IDENTIFIED FOR EXTERIOR USE THAT COMPLIES WITH THE REQUIREMENTS OF 2016 CBC SECTION 2303.2

FIRE EXTINGUISHER NOTE:

A PORTABLE FIRE EXTINGUISHER SHALL BE INSTALLED ON EACH FLOOR UNDER CONSTRUCTION PER 2013 CBC 906.1

PLUMBING NOTES:

SHOWERS ARE TO HAVE MIN. INTERIOR AREA OF 1,204 SQUARE INCHES AND SHALL BE CAPABLE OF ENCOMPASSING A 30 INCH CIRCLE PER 2016 CPC 408.6

ALL NEW PLUMBING FIXTURES SHOWN ON PLAN TO BE TIED INTO EXISTING WATER/ SEWER SYSTEMS LOCATED ON PARCEL/ NEW PLUMBING FIXTURES TO BE WATER CONSERVING DEVICES AS SPECIFIED BELOW

NON-COMPLIANT (E) PLUMBING FIXTURES WILL BE REPLACED TO COMPLY WITH CURRENT CPC REQUIREMENTS PER 2016 CCC

NON-COMPLIANT PLUMBING FIXTURE:
- WATER CLOSET MORE THAN 1.6 GALLONS/ FLUSH
- SHOWERHEAD MORE THAN 2.5 GALLONS/ MIN.
- INTERIOR FAUCET MORE THAN 2.2 GALLONS/ MINUTES

CPC REQUIREMENTS:
- WATER CLOSET: 1.28 GALLONS MAX./ FLUSH.
- SINGLE SHOWER HEAD: 1.8 GALLONS MAX./ MIN. @ 80 PSI.
- LAVATORY: 1.2 GALLONS MAX./ MIN. @ 60 PSI.
- KITCHEN FAUCET: 1.8 GALLONS MAX./ MIN. @ 60 PSI@

PROVIDE PRESSURE BALANCING VALVES AT ALL SHOWERS

NOTE:

CEILINGS AND RAFTER ROOFS THAT ARE OPENED DURING CONSTRUCTION SHALL BE INSULATED BETWEEN WOOD FRAMING AND MEMBERS WITH INSULATION IN AN INSTALLED THERMAL RESISTANCE OF R-22 OR GREATER FOR THE INSULATION ONLY PER 2016 CALIFORNIA ENERGY CODE 150.0(a)

FLOOR PLAN GENERAL NOTES:

FRENCH DOORS SHALL BE TEMPERED AND DUAL-GLAZED. (N) WINDOWS SHALL BE DUAL-GLAZED.

ALL EXTERIOR OPENINGS SHALL BE FLASHED IN ORDER TO MAKE THEM WATERPROOF.

ALL CONCEALED PLUMBING JOINTS SHALL BE NON-SLIP CONNECTIONS.

PROVIDE ACCESS PANEL (MIN. 12" X 12") OR UTILITY SPACE FOR ALL PLUMBING FIXTURES HAVING CONCEALED SLIP JOIN

PROVIDE DEVICES TO ABSORB HIGH PRESSURES RESULTING FROM THE QUICK CLOSING OF THE QUICK-ACTING VALVES FROM THE WASHER AND DISHWASHER PER C.P.C. SECTION 609.10.

WATER PRESSURE IN BUILDING SHALL BE LIMITED TO 80 P.S.I. OR

ALL PLUMBING WALLS SHALL BE 2 X 6 STUDS.

NO GAS PIPING SHALL BE INSTALLED IN OR ON THE GROUND, UNDER ANY BUILDING OR STRUCTURE. ALL EXPOSED GAS PIPING SHALL BE KEPT AT LEAST SIX (6) INCHES ABOVE GRADE OR STRUCTURE

ALL HOSE BIBS AND LAWN SPRINKLER SYSTEMS SHALL HAVE APPROVED BACKFLOW PREVENTION DEVICES. FIELD VERIFY AND RELOCATE AS REQUIRED TO ACCOMMODATE DEMOLITION AND NEW CONSTRUCTION.

WINDOW AND DOOR NOTES:

VERIFY ALL NEW & EXISTING WINDOWS AND DOORS (TYPES, ROUGH OPENING, MATERIAL, DIRECTION OF OPERABILITY, IF ANY, ETC.) WITH OWNER & ARCHITECT PRIOR TO PURCHASE AND INSTALLATION.

NFRC THERMAL PERFORMANCE LABELS SHALL REMAIN ON THE WINDOWS AND/OR DOORS UNTIL FINAL INSPECTION

ALL DOORS AND WINDOWS ARE EXISTING TO REMAIN, UNLESS NOTED OTHERWISE.

ALL NEW EXTERIOR FRENCH DOORS AND WINDOWS IN HIGH FIRE ZONE SHALL BE DUAL GLAZED W/ MIN. OF ONE TEMPERED PANE, LOW E. PER CRC R337.8.

ALL GLAZING WITHIN 18" OF FINISH FLOOR, IN ENTRY DOOR, GREATER THAN 9 SF. SHALL BE TEMPERED GLASS.

EXTERIOR DOORS SHALL COMPLY WITH ONE OF THE FOLLOWING:

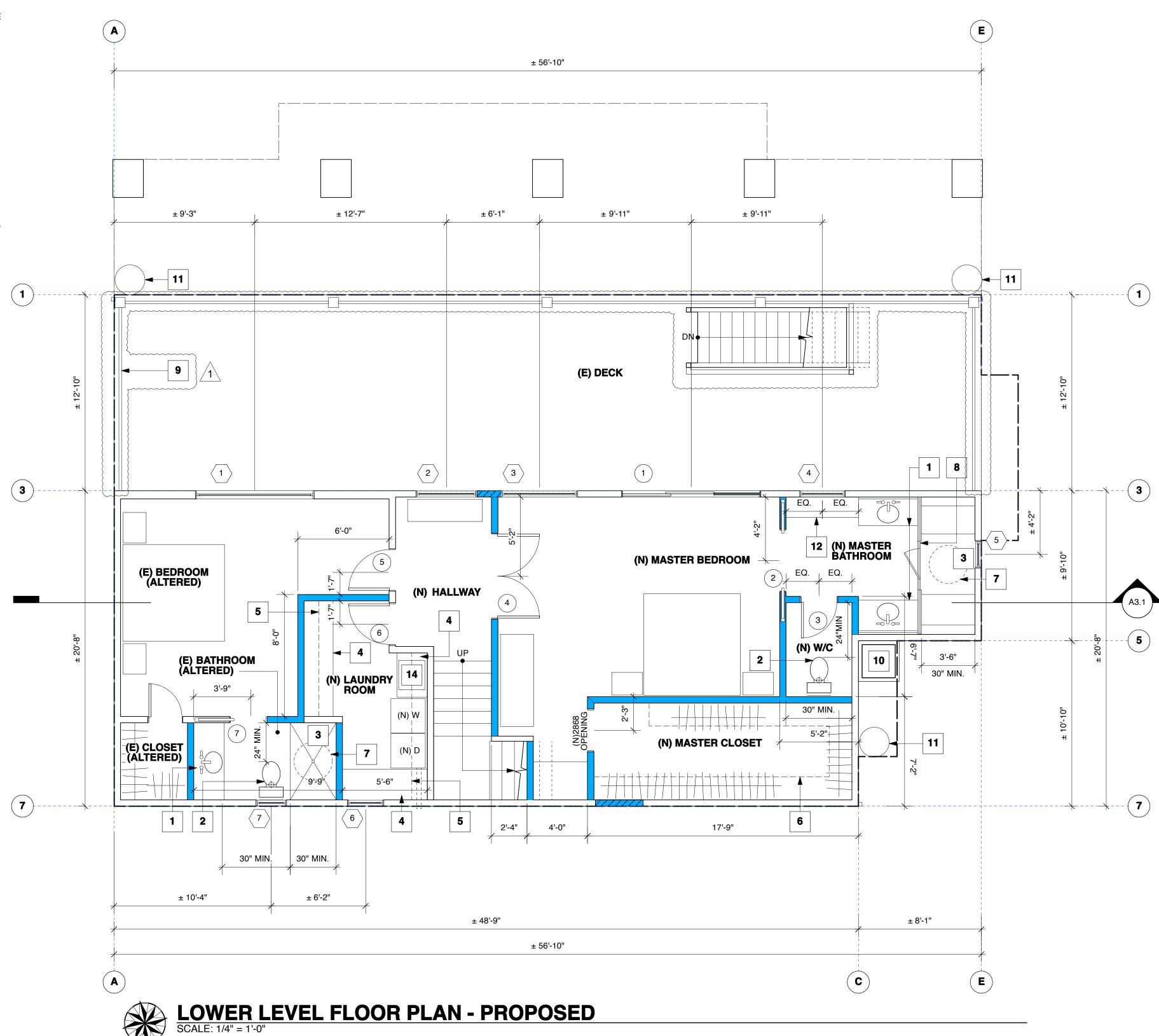
1. EXTERIOR SURFACE OR CLADDING SHALL BE OF NON-COMBUSTIBLE OR IGNITION RESISTANT MATERIAL.

2 SHALL BE CONSTRUCTED OF SOLID CORE WOOD THAT COMPLY WITH THE FOLLOWING: STILE AND RAILS SHALL NOT BE LESS THAN 1-3/8 INCHES THICK, RAISED PANELS SHALL NOT BE LESS THAN 1-1/4 INCHES THICK, EXCEPT FOR THE EXTERIOR PERIMETER OF THE RAISED PANEL THAT MAY TAPER TO A TONGUE NOT LESS THAN 3/8 INCH THICK.

3. SHALL HAVE A FIRE -RESISTANCE RATING OF NOT LESS THAN 20 MINUTES WHEN TESTED ACCORDING TO NFPA 252.

4. SHALL BE TESTED TO THE PERFORMANCE REQUIREMENTS OF SFM STANDARD 12-7A-1.

EMERGENCY ESCAPE AND RESCUE OPENINGS ARE TO HAVE A MINIMUM NET CLEAR OPENING OF NOT LESS THAN 5.7 SQ. FEET (5.0 SQ. FT. MINIMUM IF AT GRADE FLOOR LEVEL). THEY SHALL ALSO HAVE A MINIMUM CLEAR OPENING WIDTH OF 20" AND A MINIMUM CLEAR OPENING HEIGHT OF 24". WHERE A WINDOW IS PROVIDED AS THE EMERGENCY ESCAPE & RESCUE OPENING, IT SHALL HAVE THE BOTTOM OF THE CLEAR OPENING NOT GREATER THAN 44 INCHES MEASURED FROM THE FLOOR.



KEYNOTES

2 (N) TOILET

1 (N) VANITY W/ STORAGE BELOW

36"H COUNTER W/ STORAGE BELOW. VERIFY FINISH W/ OWNER

(N) UPPER CABINET

(N) RODS AND SHELF

30" DIA. CLEARANCE

(N) SHOWER

11 (N) RAIL BARREL, CONNECT (E) DOWNSPOUT, REFER TO DETAIL 3/A5.3

12 (N) MAKEUP COUNTER PER OWNER

13 NOT USED

14 (N) SINK

(N) 1/2" SEAMLESS SHATTERPROOF 2'-0" GLASS DOOR

(E) METAL RAILING TO BE REPLACED WITH (N) CABLE RAILING, **REFER TO DETAIL 4,5/A5.3**

A/C CONDENSER & PAD, REFER TO DETAIL 2/A5.3

WALL LEGEND

(E) WALL TO REMAIN

(E) WALL TO REMOVED

(N) PARTITION WALL - REFER TO 2/A 5.1

(N) 1-HR EXTERIOR WALL - REFER TO 1/A 5.1

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10/31/19 SFDB REVISION #1 SUBMITTAL

DATE DESCRIPTION

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AND EXTERIOR ALTERATIONS SINGLE FAMILY RESIDENCE

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LOWER LEVEL FLOOR PLAN - PROPOSED

FLOOR PLAN GENERAL NOTES: FRENCH DOORS SHALL BE TEMPERED AND DUAL-GLAZED. (N) WINDOWS SHALL BE DUAL-GLAZED. ALL EXTERIOR OPENINGS SHALL BE FLASHED IN ORDER TO MAKE THEM WATERPROOF. ALL CONCEALED PLUMBING JOINTS SHALL BE NON-SLIP CONNECTIONS. PROVIDE ACCESS PANEL (MIN. 12" X 12") OR UTILITY SPACE FOR ALL PLUMBING FIXTURES HAVING CONCEALED SLIP JOIN PROVIDE DEVICES TO ABSORB HIGH PRESSURES RESULTING FROM THE QUICK CLOSING OF THE QUICK-ACTING VALVES FROM THE WASHER AND DISHWASHER PER C.P.C. SECTION 609.10. WATER PRESSURE IN BUILDING SHALL BE LIMITED TO 80 P.S.I. OR ALL PLUMBING WALLS SHALL BE 2 X 6 STUDS. NO GAS PIPING SHALL BE INSTALLED IN OR ON THE GROUND, UNDER ANY BUILDING OR STRUCTURE. ALL EXPOSED GAS PIPING SHALL BE KEPT AT LEAST SIX (6) INCHES ABOVE GRADE

RELOCATE AS REQUIRED TO ACCOMMODATE DEMOLITION AND NEW CONSTRUCTION.

WINDOW AND DOOR NOTES:

ALL HOSE BIBS AND LAWN SPRINKLER SYSTEMS SHALL HAVE

OR STRUCTURE.

VERIFY ALL NEW & EXISTING WINDOWS AND DOORS (TYPES, ROUGH OPENING, MATERIAL, DIRECTION OF OPERABILITY, IF ANY, ETC.) WITH OWNER & ARCHITECT PRIOR TO PURCHASE AND INSTALLATION.

NFRC THERMAL PERFORMANCE LABELS SHALL REMAIN ON THE WINDOWS AND/OR DOORS UNTIL FINAL INSPECTION

ALL DOORS AND WINDOWS ARE EXISTING TO REMAIN, UNLESS NOTED OTHERWISE.

ALL NEW EXTERIOR FRENCH DOORS AND WINDOWS IN HIGH FIRE ZONE SHALL BE DUAL GLAZED W/ MIN. OF ONE TEMPERED PANE, LOW E. PER CRC R337.8.

ALL GLAZING WITHIN 18" OF FINISH FLOOR, IN ENTRY DOOR, GREATER THAN 9 SF. SHALL BE TEMPERED GLASS.

EXTERIOR DOORS SHALL COMPLY WITH ONE OF THE FOLLOWING:

1. EXTERIOR SURFACE OR CLADDING SHALL BE OF NON-COMBUSTIBLE OR IGNITION RESISTANT MATERIAL.

2 SHALL BE CONSTRUCTED OF SOLID CORE WOOD THAT COMPLY WITH THE FOLLOWING: STILE AND RAILS SHALL NOT BE LESS THAN 1-3/8 INCHES THICK, RAISED PANELS SHALL NOT BE LESS THAN 1-1/4 INCHES THICK, EXCEPT FOR THE EXTERIOR PERIMETER OF THE RAISED PANEL THAT MAY TAPER TO A TONGUE NOT LESS THAN 3/8 INCH THICK.

3. SHALL HAVE A FIRE -RESISTANCE RATING OF NOT LESS THAN 20 MINUTES WHEN TESTED ACCORDING TO NFPA 252.

4. SHALL BE TESTED TO THE PERFORMANCE REQUIREMENTS OF SFM STANDARD 12-7A-1.

EMERGENCY ESCAPE AND RESCUE OPENINGS ARE TO HAVE A MINIMUM NET CLEAR OPENING OF NOT LESS THAN 5.7 SQ. FEET (5.0 SQ. FT. MINIMUM IF AT GRADE FLOOR LEVEL). THEY SHALL ÀLSO HAVE A MINIMUM CLEAR OPENING WIDTH ÓF 20" AND A MINIMUM CLEAR OPENING HEIGHT OF 24". WHERE A WINDOW IS PROVIDED AS THE EMERGENCY ESCAPE & RESCUE OPENING, IT SHALL HAVE THE BOTTOM OF THE CLEAR OPENING NOT GREATER THAN 44 INCHES MEASURED FROM THE FLOOR.

FIRE NOTES:

ALL ELEMENTS OF NEW AND RE-BUILT CONSTRUCTION MUST COMPLY WITH REQUIREMENTS SET FORTH IN SECTION R337 OF THE 2016 CRC AND S.B. ORDINANCE NO. 5780.

1. ALL NEW ROOF UNDERLAYMENT TO BE CLASS A - TARCO LEAKBARRIER EASYSTICK PLUS AND EASYBASE MEMBRANE (ESR-3297) OR EQUIVALENT.

2. ALL NEW EXTERIOR WALLS TO BE 1-HR RATED. - REFER TO DETAIL 1/A 5.1.

3. ALL NEW ROOF SHEATHING ON THE UNDERSIDE OF UNENCLOSED ROOF EAVES TO CONSIST OF 5/8" TYPE 'X' GYPSUM BOARD. - CERTAIN TEED CORP. 5/8" TYPE 'X' EXTERIOR SOFFIT BOARD OR EQUIVALENT.

4. ALL EXPOSED WOOD TO BE FIRE-RETARDANT-TREATED WOOD*. - HOOVER TREATED LUMBER W/ EXTERIOR FIRE X OR

*FIRE-RETARDANT-TREATED WOOD IDENTIFIED FOR EXTERIOR USE THAT COMPLIES WITH THE REQUIREMENTS OF 2016 CBC SECTION 2303.2

FIRE EXTINGUISHER NOTE:

A PORTABLE FIRE EXTINGUISHER SHALL BE INSTALLED ON EACH FLOOR UNDER CONSTRUCTION PER 2013 CBC 906.1

PLUMBING NOTES:

SHOWERS ARE TO HAVE MIN. INTERIOR AREA OF 1,204 SQUARE INCHES AND SHALL BE CAPABLE OF ENCOMPASSING A 30 INCH CIRCLE PER 2016 CPC 408.6

ALL NEW PLUMBING FIXTURES SHOWN ON PLAN TO BE TIED INTO EXISTING WATER/ SEWER SYSTEMS LOCATED ON PARCEL/ NEW PLUMBING FIXTURES TO BE WATER CONSERVING DEVICES AS

NON-COMPLIANT (E) PLUMBING FIXTURES WILL BE REPLACED TO COMPLY WITH CURRENT CPC REQUIREMENTS PER 2016 CCC

- NON-COMPLIANT PLUMBING FIXTURE: - WATER CLOSET MORE THAN 1.6 GALLONS/ FLUSH
- SHOWERHEAD MORE THAN 2.5 GALLONS/ MIN. - INTERIOR FAUCET MORE THAN 2.2 GALLONS/ MINUTES
- CPC REQUIREMENTS:
- WATER CLOSET: 1.28 GALLONS MAX./ FLUSH. - SINGLE SHOWER HEAD: 1.8 GALLONS MAX./ MIN. @ 80 PSI.
- LAVATORY: 1.2 GALLONS MAX./ MIN. @ 60 PSI. - KITCHEN FAUCET: 1.8 GALLONS MAX./ MIN. @ 60 PSI@

PROVIDE PRESSURE BALANCING VALVES AT ALL SHOWERS

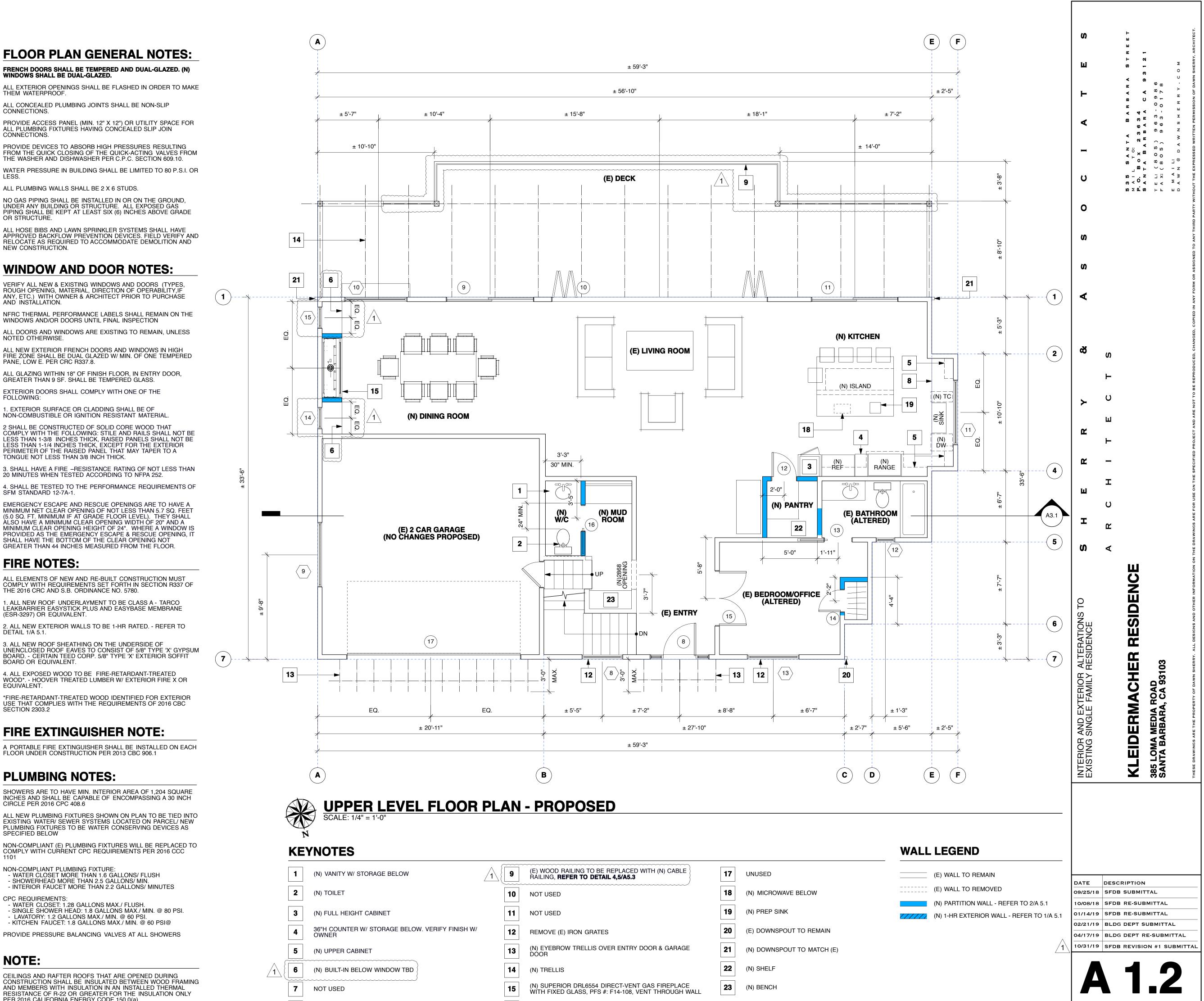
NOTE:

CEILINGS AND RAFTER ROOFS THAT ARE OPENED DURING CONSTRUCTION SHALL BE INSULATED BETWEEN WOOD FRAMING AND MEMBERS WITH INSULATION IN AN INSTALLED THERMAL RESISTANCE OF R-22 OR GREATER FOR THE INSULATION ONLY PER 2016 CALIFORNIA ENERGY CODE 150.0(a)

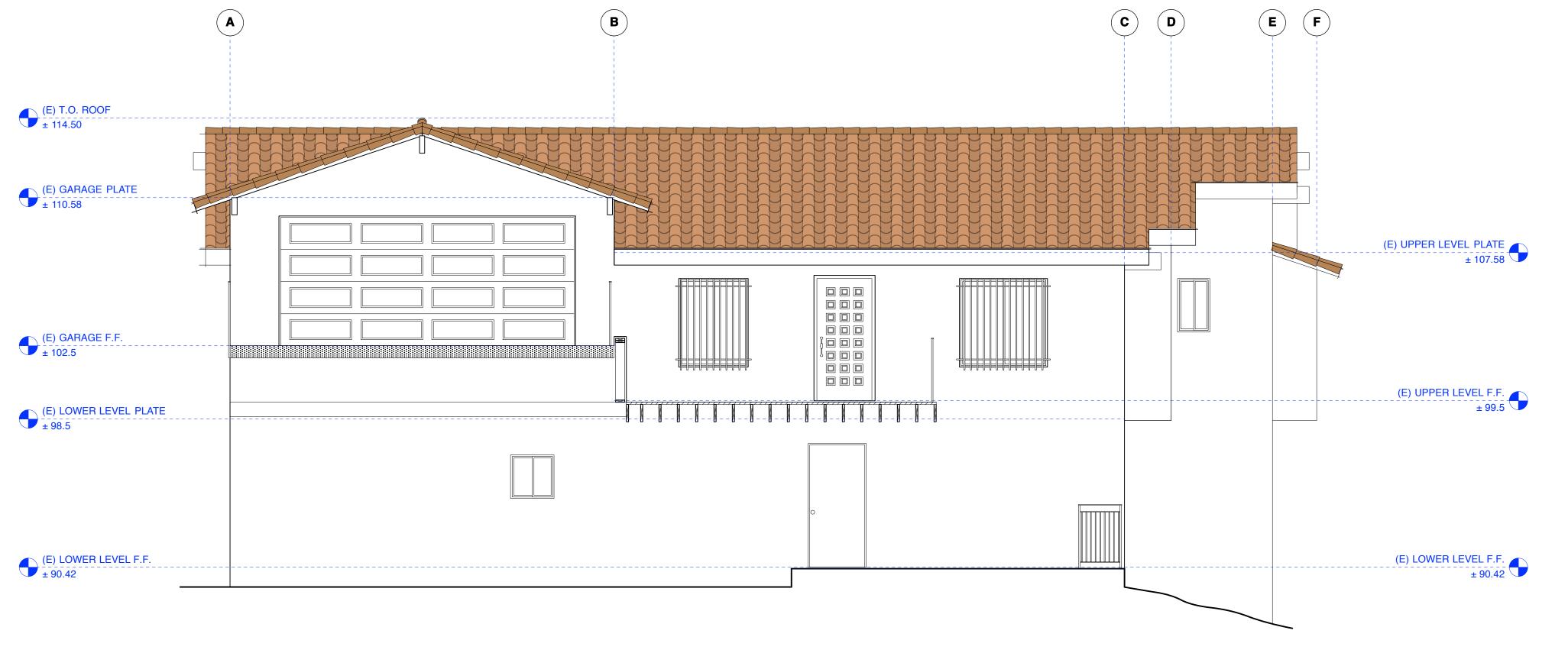
36"H COUNTER W/ DOUBLE SINKS AND STORAGE BELOW.

VERIFY FINISH W/ OWNER

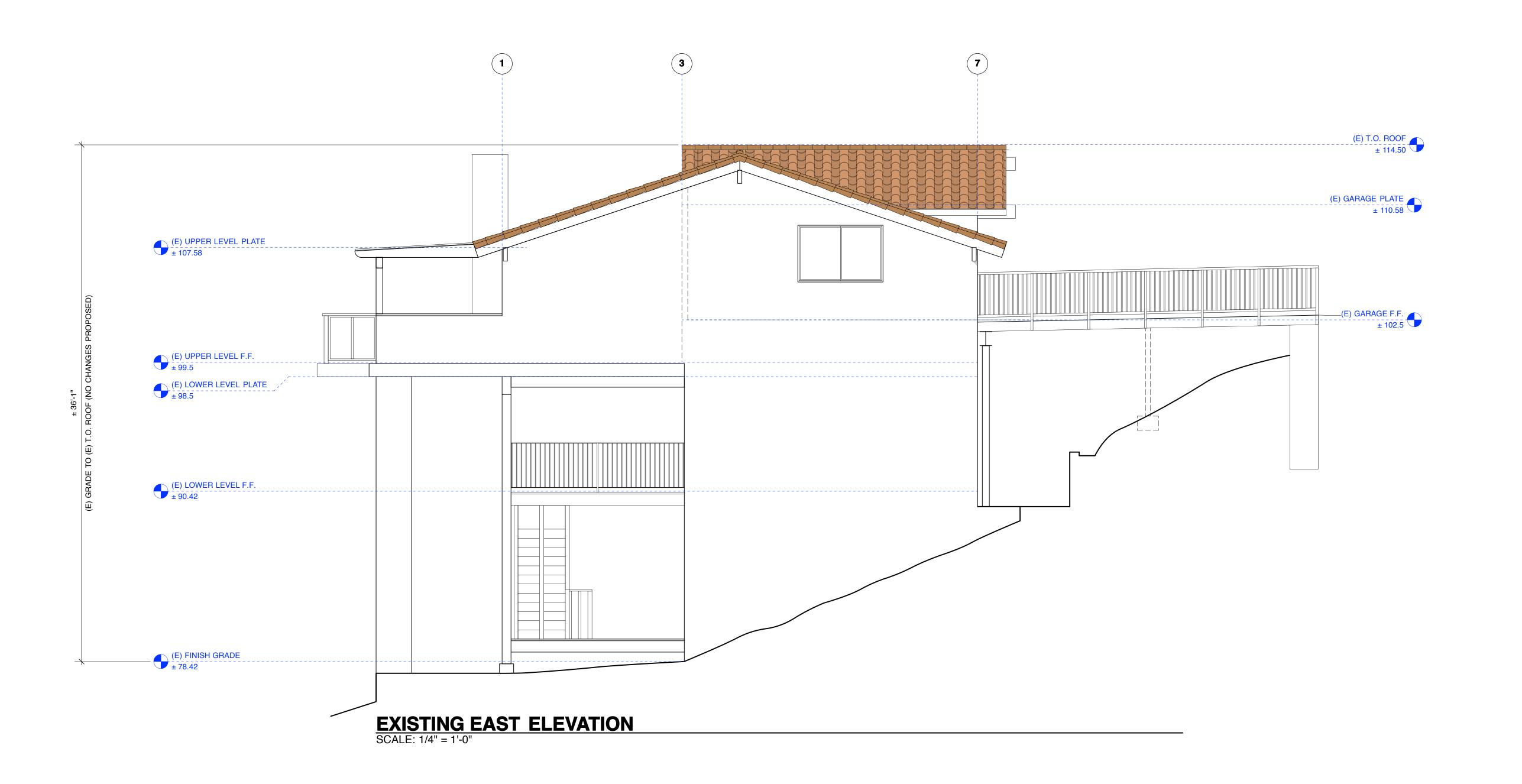
16 NOT USED

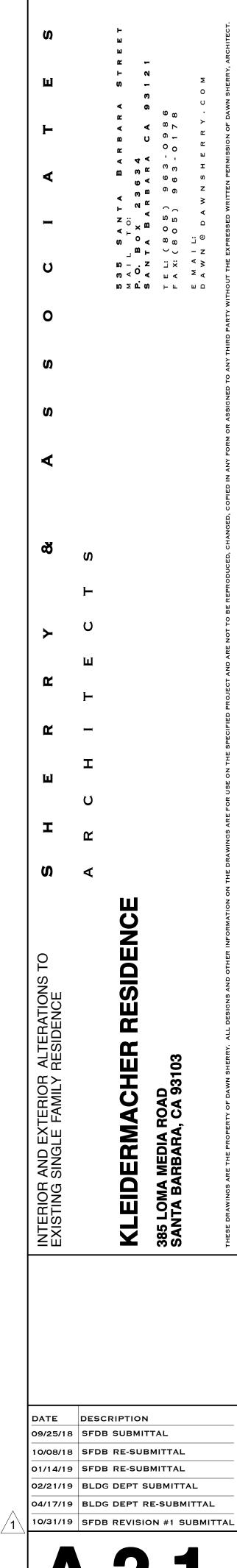


UPPER LEVEL FLOOR PLAN - PROPOSED

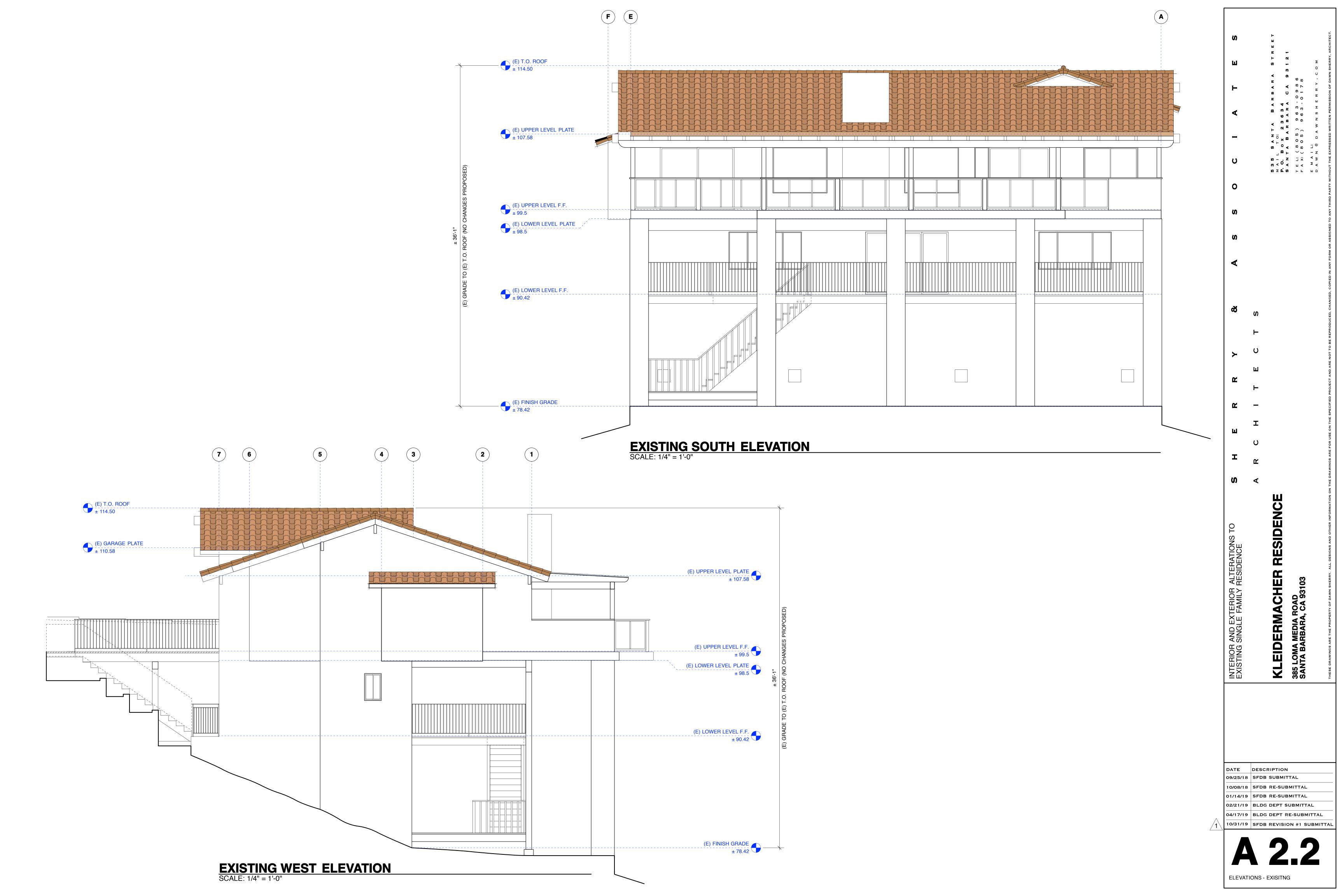


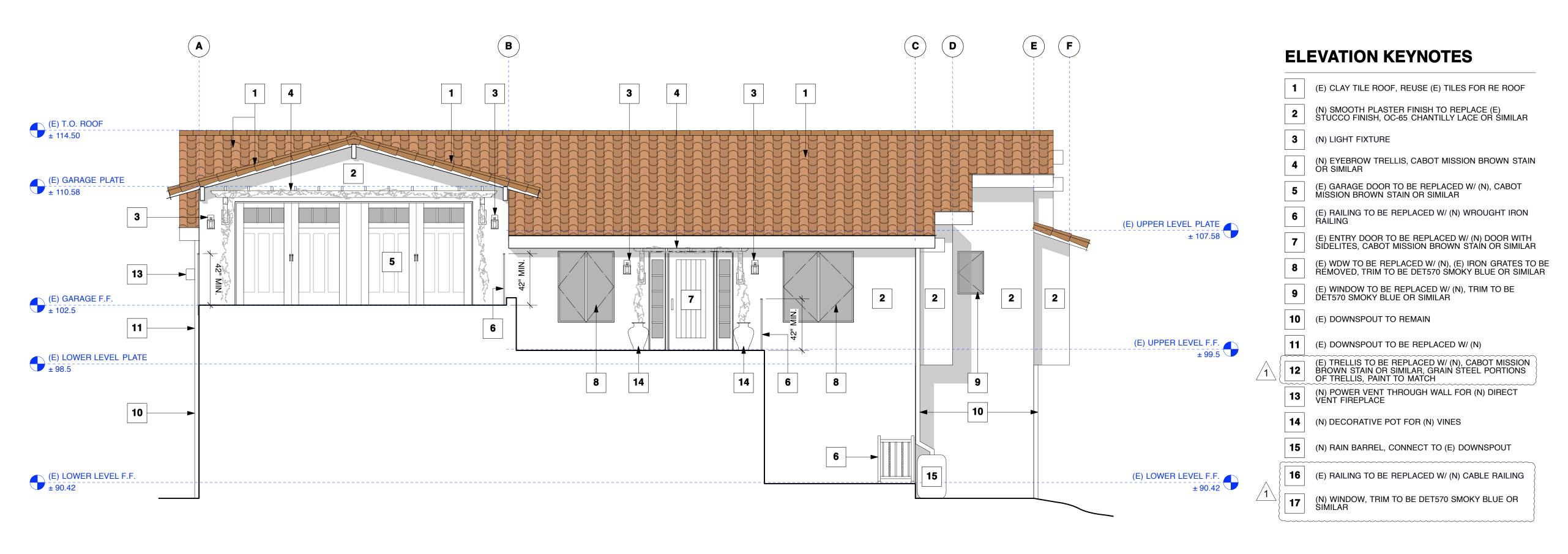
EXISTING NORTH ELEVATION SCALE: 1/4" = 1'-0"



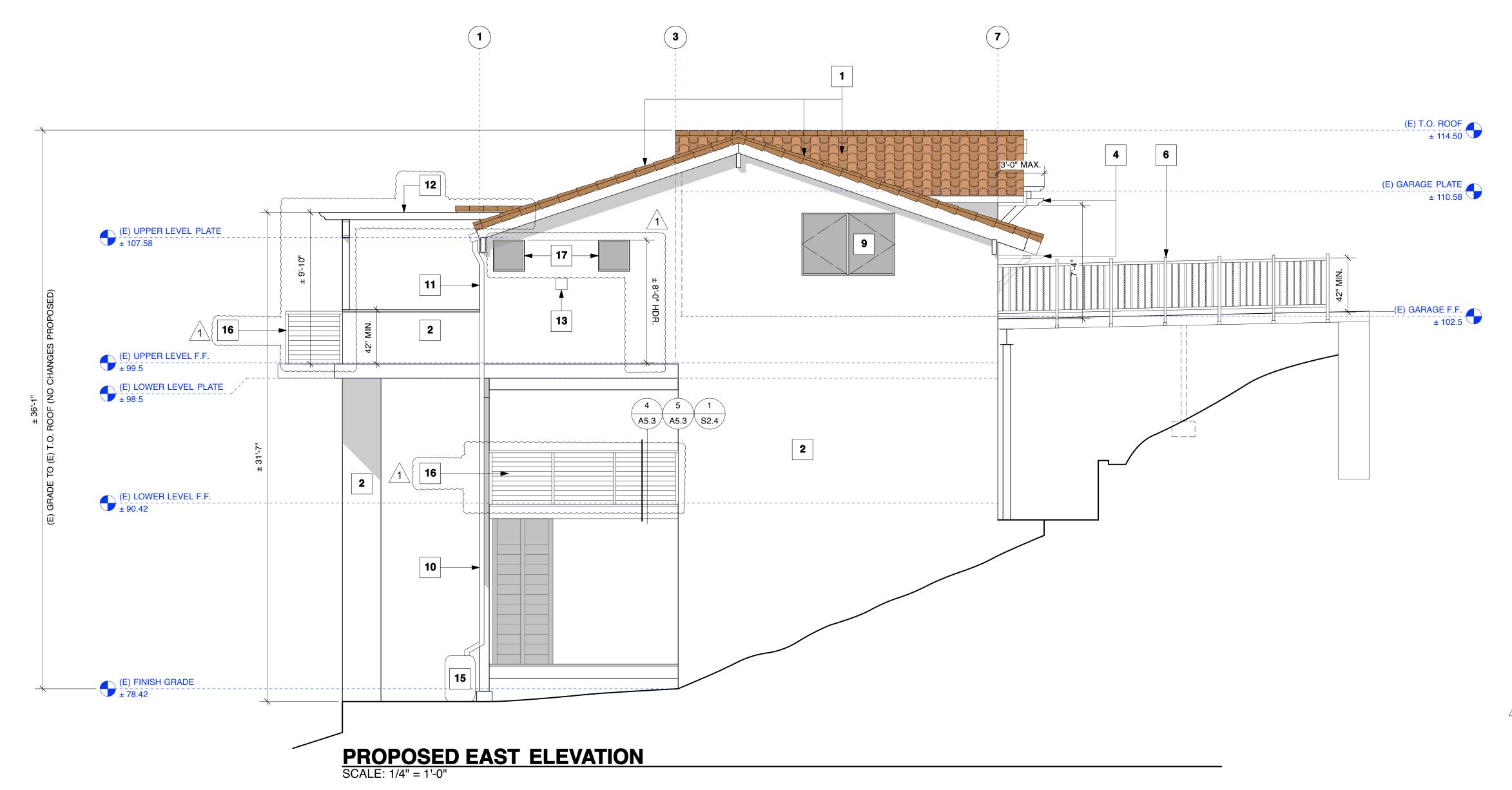


ELEVATIONS - EXISITNG





PROPOSED NORTH ELEVATION SCALE: 1/4" = 1'-0"



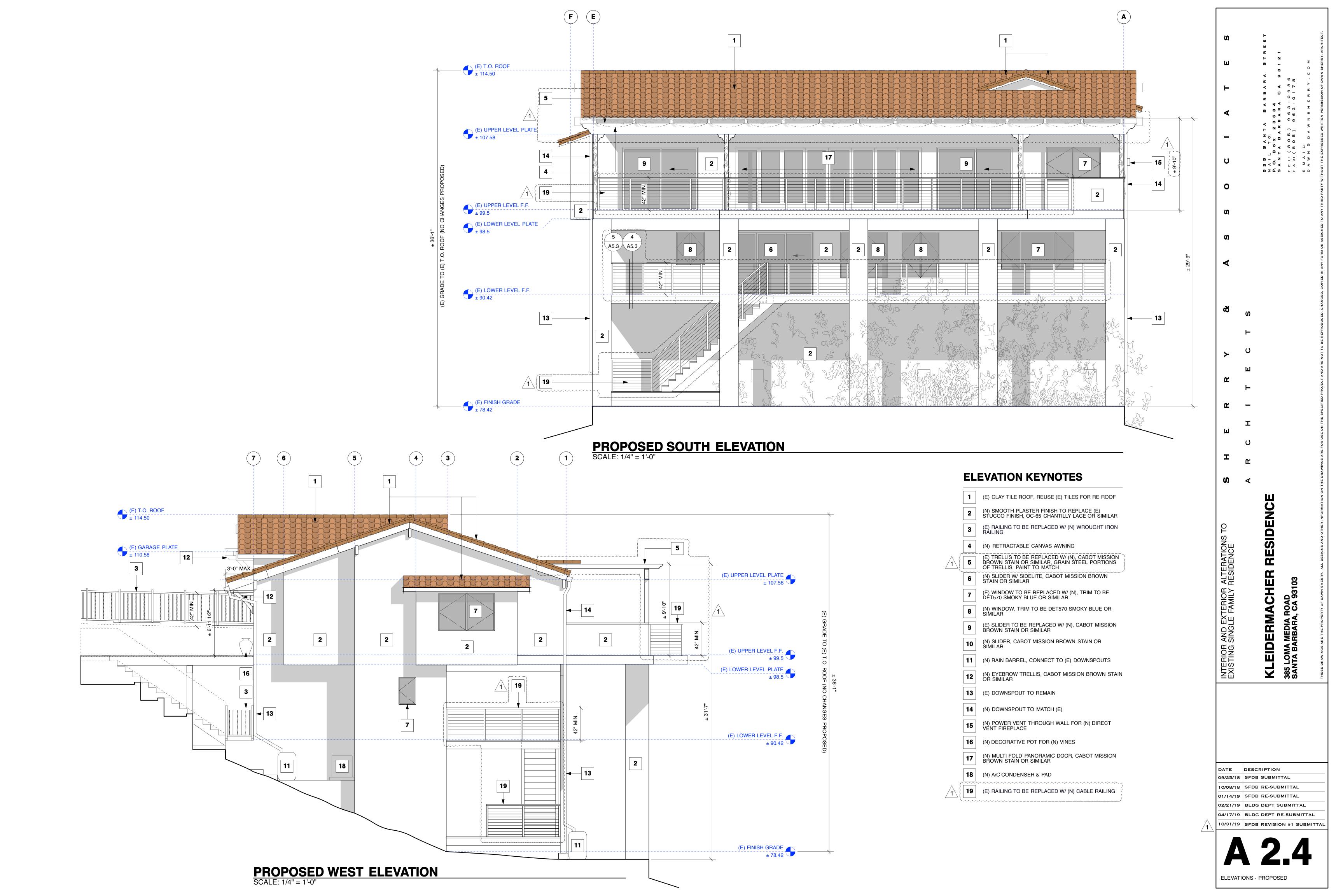
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10/08/18 SFDB RE-SUBMITTAL

01/14/19 SFDB RE-SUBMITTAL 02/21/19 BLDG DEPT SUBMITTAL 04/17/19 BLDG DEPT RE-SUBMITTAL

10/31/19 SFDB REVISION #1 SUBMITTAL

ELEVATIONS - PROPOSED



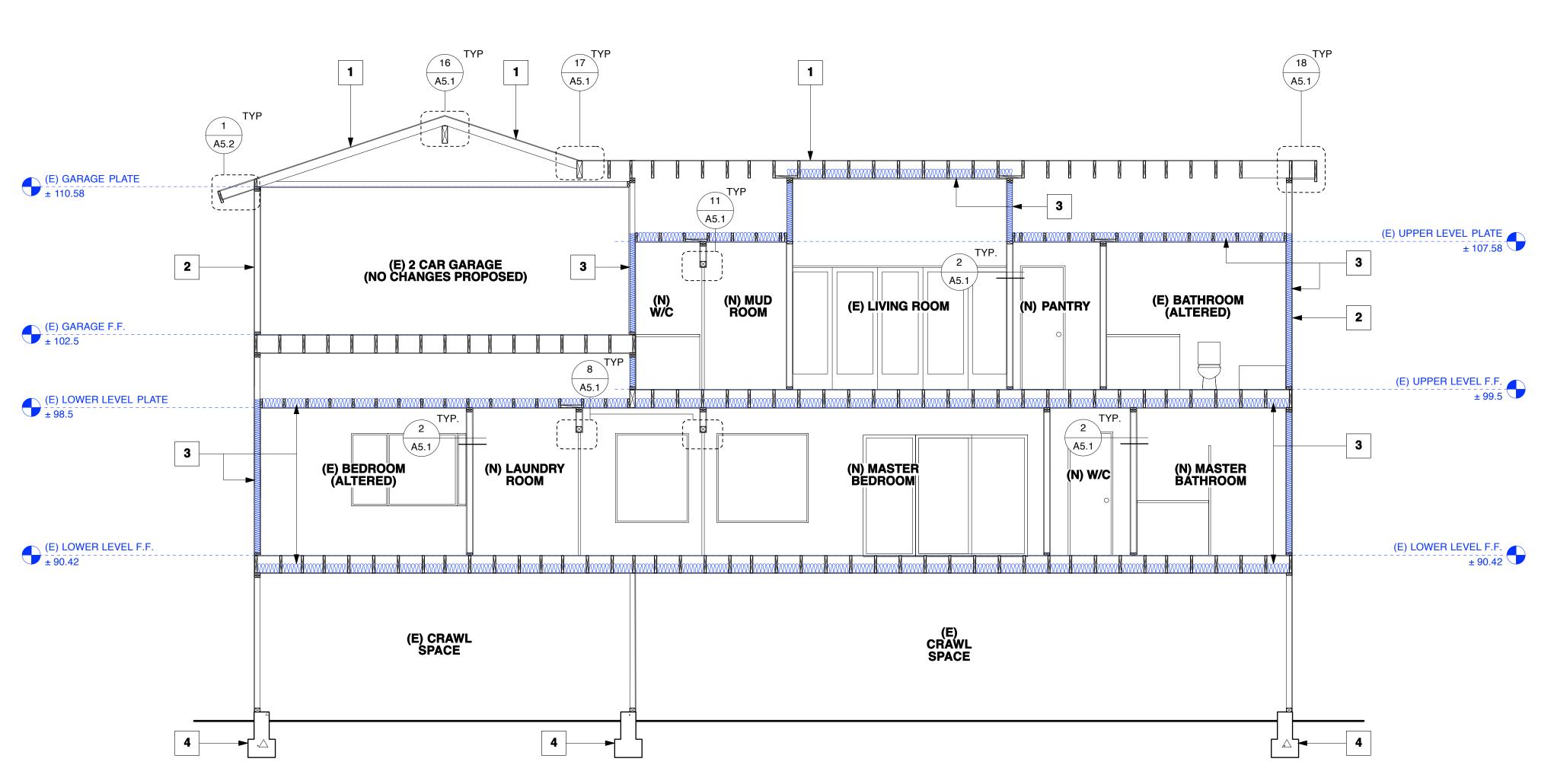
SECTION KEYNOTES

(E) CLAY TILE ROOF, REUSE (E) TILES FOR RE ROOF

(N) SMOOTH PLASTER FINISH TO REPLACE (E) STUCCO FINISH

3 (E) INSULATION TO REMAIN

4 (E) FOOTING TO REMAIN



PROPOSED SECTION
SCALE: 1/4" = 1'-0"

535 SANTA BARBAR
M A I L TO:
P.O. BOX 23634
SANTA BARBARA CA
TEL: (805) 963-0986
FAX: (805) 963-0178
E MAIL:
DAWN @ DAWNSHERRY. ಹ Ø ∢ RESIDENCE INTERIOR AND EXTERIOR ALTERATIONS TO EXISTING SINGLE FAMILY RESIDENCE KLEIDERMACHER 385 LOMA MEDIA ROAD SANTA BARBARA, CA 93103 DATE DESCRIPTION 09/25/18 | SFDB SUBMITTAL 10/08/18 SFDB RE-SUBMITTAL

01/14/19 SFDB RE-SUBMITTAL 02/21/19 BLDG DEPT SUBMITTAL

04/17/19 BLDG DEPT RE-SUBMITTAL 10/31/19 SFDB REVISION #1 SUBMITTAL

PROPOSED SECTION

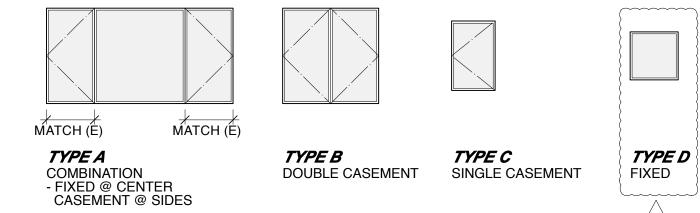
WINDOW SCHEDULE

WINDOW L	OCATION		WINDOW ROUG	H OPENING					GLASS		WINDOW F	RAME	WINDOW	DETAILS	
		ROOM	WIDTH (R.O.)	HEIGHT (R.O.)	TYPE REMARKS	U FACTOR	SHGC	EGRESS-UBC 1204	DOUBLE GLAZING	TEMPERED	MATERIAL	FINISH COLOR	HEAD	JAMB	SILL
1 L	OWER LEVEL	(E) BEDROOM (ALTER)	8'-0" W.	4'-0" H.	A (N) WINDOW TO REPLACE (E) WINDOW	0.32	0.25	YES	YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
2		(N) HALLWAY	4'-0" W.	5'-0" H.	B (N) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
3		(N) MASTER BEDROOM	5'-0" W.	5'-0" H.	B (N) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
4		(N) MASTER BATHROOM	3'-0" W.	3'-6" H.	B (N) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
5		(N) MASTER BATHROOM	2'-0" W.	3'-0" H.	C (N) WINDOW TO REPLACE (E) WINDOW, RATED FOR WET CONDITIONS	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
6		(N) LAUNDRY ROOM	2'-0" W.	3'-0" H.	C (N) WINDOW TO REPLACE (E) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
7		(E) BATHROOM (ALTER)	2'-0" W.	3'-0" H.	C (N) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
8 U	JPPER LEVEL	(E) ENTRY	4'-0" W.	5'-0" H.	B (N) WINDOW TO REPLACE (E) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
9		(E) GARAGE	6'-0" W.	4'-0" H.	B (N) WINDOW TO REPLACE (E) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
10		(N) DINING ROOM	6'-0" W.	3'-8" H	B (N) WINDOW TO REPLACE (E) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
11		(N) KITCHEN	6'-0" W.	4'-0" H.	B (N) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
12		(E) BATHROOM (ALTER)	2'-0" W.	3'-0" H	C (N) WINDOW TO REPLACE (E) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
13	~~~~~	(E) BEDROOM/OFFICE (ALTER	R) 5'-0" W.	5'-0" H.	B (N) WINDOW TO REPLACE (E) WINDOW	0.32	0.25	YES	YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
14		(N) DINING ROOM	2'-0" W.	2'-0" H.	D (N) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
15		(N) DINING ROOM	2'-0" W.	2'-0" H.	D (N) WINDOW	0.32	0.25		YES	YES	VINYL	PAINTED DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1

WINDOW SCHEDULE - ALTERNATES

WINDOW LOCATION		WINDOW ROUG	GH OPENING	1					GLASS		WINDOW F	RAME		WINDOW	DETAILS	
	ROOM	WIDTH (R.O.)	HEIGHT (R.	O.) TYPE	REMARKS	U FACTOR	SHGC	EGRESS-UBC 1204	DOUBLE GLAZING	TEMPERED	MATERIAL	FINISH	COLOR	HEAD	JAMB	SILL
1A UPPER LEVEL	(E) LIVING ROOM	6'-0" W.	5'-0" H.	В	(N) WINDOW TO REPLACE (E) WINDOW	0.32	0.25	5	YES	YES	VINYL	PAINTED	DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
2A	(E) LIVING ROOM	6'-0" W.	5'-0" H.	В	(N) WINDOW	0.32	0.25	5	YES	YES	VINYL	PAINTED	DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1
ЗА	(E) LIVING ROOM	6'-0" W.	5'-0" H.	В	(N) WINDOW TO REPLACE (E) WINDOW	0.32	0.2	5	YES	YES	VINYL	PAINTED	DET570 SMOKY BLUE OR SIMILAR	5/A 5.1	4/A 5.1	6/A 5.1

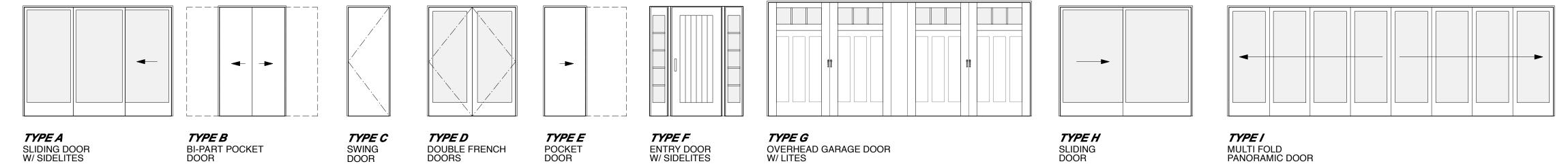
WINDOW TYPES



DOOR SCHEDULE

DOOR LOCATION		DOOR OPENING		DOOR			GLASS		DOOR		DOOR FRA	AME		DOOR DETA	AILS	
	ROOM	WIDTH	HEIGHT	TYPE	DOOR DESCRIPTION	U FACTOR SHGC	DOUBLE GLAZING	TEMPERED	MATERIAL	FINISH	MATERIAL	FINISH	COLOR	HEAD	JAMB	THRESHOLD
1 EXTERIOR	(N) MASTER BEDROOM	9'-0" W.	6'-8" H.	Α	(N) 6'-0" SLIDER W/ SIDELITE		YES	YES	WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	5/A 5.1	4/A 5.1	7/A 5.1
LOWER LEVEL																
2 INTERIOR	(N) MASTER BATHROOM	4'-0" W.	6'-8" H.	В	(N) BI-PART POCKET DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	11/A 5.1	12/A 5.1	13/A 5.1
3 LOWER LEVEL	(N) MASTER BATHROOM W/C	2'-4" W.	6'-8" H.	С	(N) SWING DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	8/A 5.1	9/A 5.1	
4	(N) MASTER BEDROOM	5'-6" W.	6'-8" H.	D	(N) DOUBLE FRENCH DOOR		YES	YES	WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	8/A 5.1	9/A 5.1	
5	(E) BEDROOM (ALTER)	2'-8" W.	6'-8" H.	С	(N) SWING DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	8/A 5.1	9/A 5.1	
6	(N) LAUNDRY ROOM	2'-8" W.	6'-8" H.	С	(N) SWING DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	8/A 5.1	9/A 5.1	
7	(E) BATHROOM (ALTER)	2'-4" W.	6'-8" H.	Е	(N) POCKET DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	11/A 5.1	12/A 5.1	13/A 5.1
8 EXTERIOR	(E) ENTRY	5'-4" W.	6'-8" H.	F	(N) 3'-0" ENTRY DOOR W/ 0954 SIDELITES @ EACH SIDE		YES	YES	WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	5/A 5.1	4/A 5.1	7/A 5.1
9 UPPER LEVEL	(N) DINING ROOM	8'-0" W.	6'-8" H.	Н	(N) SLIDER TO REPLACE (E) SLIDER		YES	YES	WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	5/A 5.1	4/A 5.1	7/A 5.1
10	(E) LIVING ROOM	19'-10" W.	6'-8" H.	I	(N) MULTI-FOLD PANORAMIC DOOR		YES	YES	WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	5/A 5.1	4/A 5.1	7/A 5.1
11	(N) KITCHEN	8'-0" W.	6'-8" H.	Н	(N) SLIDER TO REPLACE (E) SLIDER		YES	YES	WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	5/A 5.1	4/A 5.1	7/A 5.1
12 INTERIOR	(N) PANTRY	2'-3" W.	6'-8" H.	С	(N) SWING DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	8/A 5.1	9/A 5.1	
13 UPPER LEVEL	(E) BATHROOM (ALTER)	2'-6" W.	6'-8" H.	E	(N) POCKET DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	11/A 5.1	12/A 5.1	13/A 5.1
14	(E) BEDROOM/OFFICE CLOSET (ALTER)	2'-0" W.	6'-8" H.	С	(N) SWING DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	8/A 5.1	9/A 5.1	
15		5'-0" W.	6'-8" H.	D	(N) DOUBLE FRENCH DOOR		YES	YES	WOOD	STAINED		T	CABOT MISSION BROWN OR SIMILAR		9/A 5.1	
16	(N) W/C	2'-4" W.	6'-8" H.	E	(N) POCKET DOOR				WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	11/A 5.1	12/A 5.1	13/A 5.1
17 GARAGE	(E) GARAGE	16'-0" W.	7'-0" H.	G	(N) OH GARAGE DOOR TO REPLACE (E)		YES	YES	WOOD	STAINED	WOOD	STAINED	CABOT MISSION BROWN OR SIMILAR	14/A 5.1	15/A 5.1	

DOOR TYPES



NOTES:

1. CONTRACTOR SHALL VERIFY ALL ROUGH OPENINGS PRIOR TO ORDERING WINDOWS AND DOORS.

2. CONTRACTOR SHALL CONFIRM DIRECTION OF OPERABILITY W/ OWNER & ARCHITECT PRIOR TO O PURCHASE AND INSTALLATION.

3. NFRC THERMAL PERFORMANCE LABELS SHALL REMAIN ON THE WINDOWS AND/OR DOORS UNTIL FINAL INSPECTION

DATE DESCRIPTION

09/25/18 SFDB SUBMITTAL

10/08/18 SFDB RE-SUBMITTAL

01/14/19 SFDB RE-SUBMITTAL

02/21/19 BLDG DEPT SUBMITTAL

04/17/19 BLDG DEPT RE-SUBMITTAL

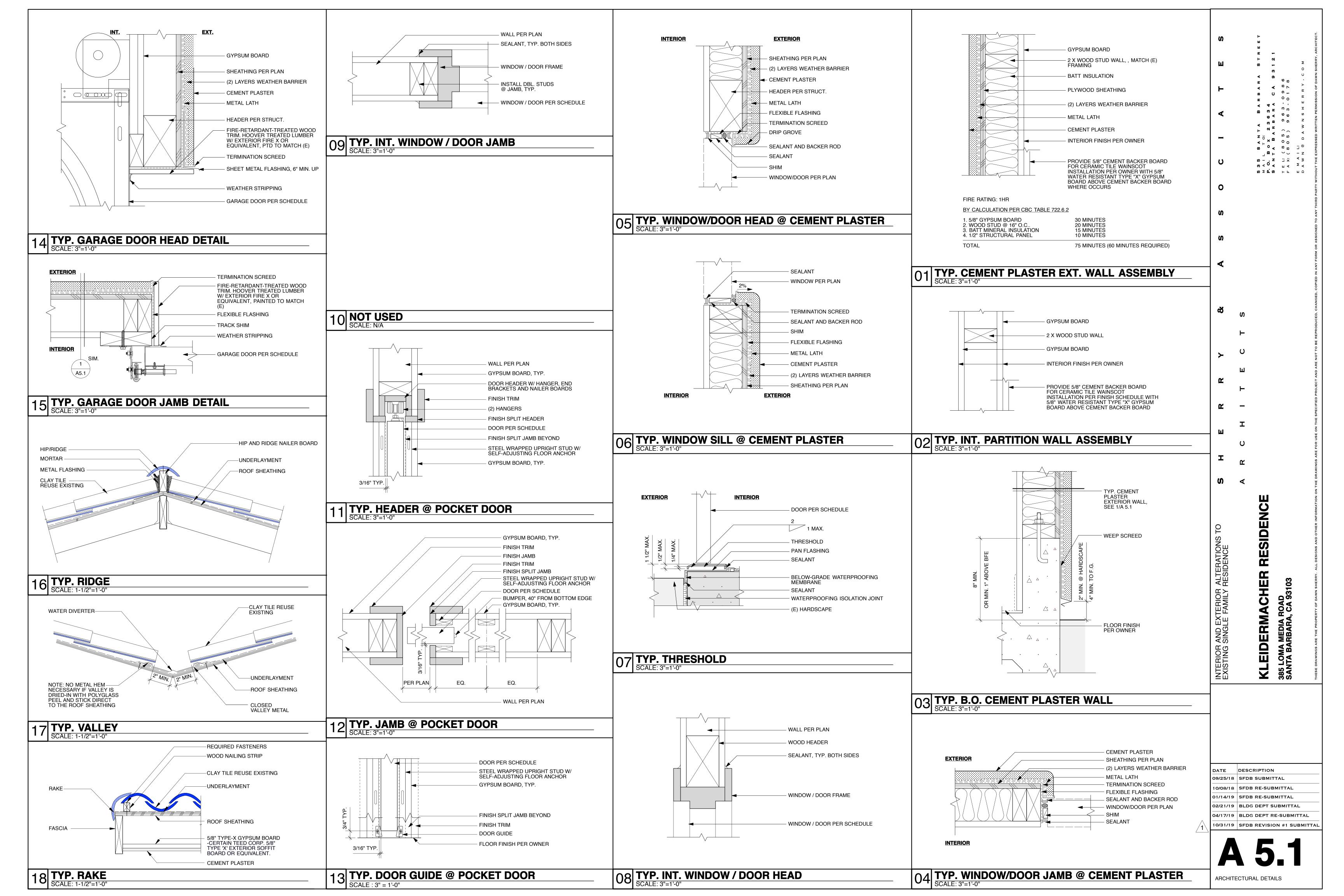
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INTERIOR AND EXTERIOR ALTERATIONS TO EXISTING SINGLE FAMILY RESIDENCE

KLEIDERMACHER

DOOR / WINDOW SCHEDULES



CALIFORNIA DEPARTMENT OF FORESTRY & FIRE PROTECTION OFFICE OF THE STATE FIRE MARSHAL FIRE ENGINEERING - BUILDING MATERIALS LISTING PROGRAM

LISTING SERVICE

CATEGORY: 2520 -- TREATED LUMBER

LISTEE: HOOVER TREATED WOOD PRODUCTS, INC.154 WIRE ROAD, THOMSON, GA 30824 Contact: Chris Athari (706) 755-5350 Fax (706) 595-6600

Email: cathari@frtw.com

DESIGN: Treated Lumber with EXTERIOR FIRE X. Products are pressure impregnated with "EXTERIOR FIRE X" fire retardant chemical. EXTERIOR FIRE X covers the following types of lumber: Douglas Fir, Southern Yellow Pine, Western Red Cedar, and Redwood Lumber. Refer to listee's printed installation instructions for additional detailed product description and

RATING: Douglas Fir: 15 Flame Spread; 30 Smoke Developed

Southern Yellow Pine:15 Flame Spread; 50 Smoke Developed Western Red Cedar:10 Flame Spread; 30-105 Smoke Developed

Redwood:10 Flame Spread; 10-115 Smoke Developed

INSTALLATION: In accordance with listee's printed installation instructions, applicable codes & ordinances and in a manner acceptable to the authority having jurisdiction.

MARKING:

APPROVAL:

LISTING No.

Listee's name, product identification, classification, and Underwriter's Laboratory label. Listed as exterior type fire retardant treated lumber. Also approved for use as ignition resistant materials as specified in Chapter 7A of the *California Building Code. The lumber

must be pressure treated with the fire retardant chemical at the factory under authorized

Corrected 9-25-12 il

Page 1 of 1

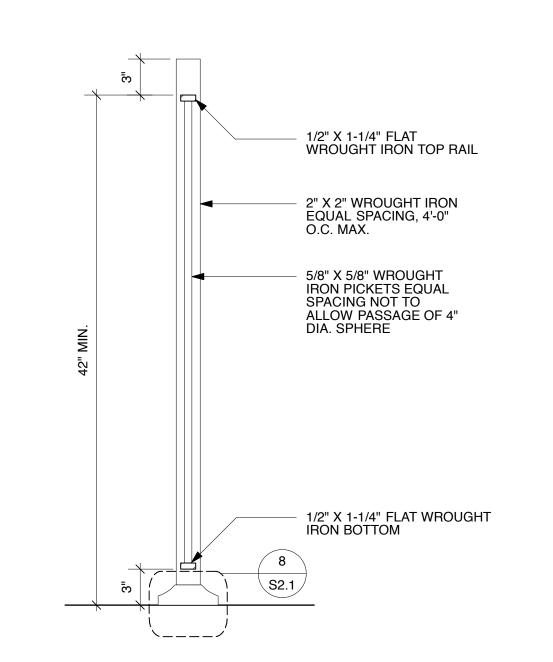
This listing is based upon technical data submitted by the applicant. CSFM Fire Engineering staff has reviewed the test results and/or other data but does not make an independent verification of any claims. This listing is not an endorsement or recommendation of the item listed. This listing should not be used to verify correct operational requirements or installation criteria. Refer to listee's data sheet, installation instructions and/or other

Listing Expires June 30, 2019 July 01, 2018 Date Issued:

Authorized By: DAVID CASTILLO, Program Coordinator

Fire Engineering Division

08 FIRE-RETARDANT-TREATED LUMBER LISTING



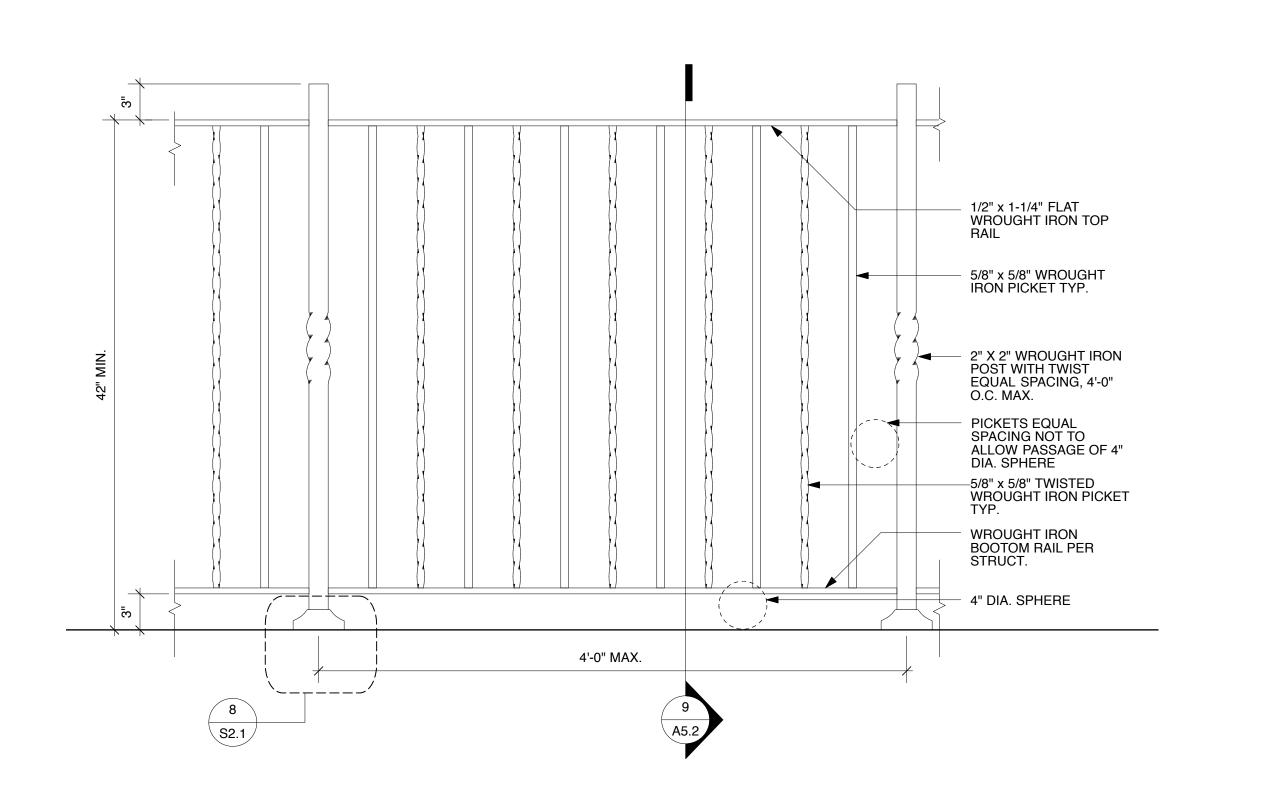
09 TYP. GUARDRAIL SECTION SCALE: 1-1/2" = 1'-0"

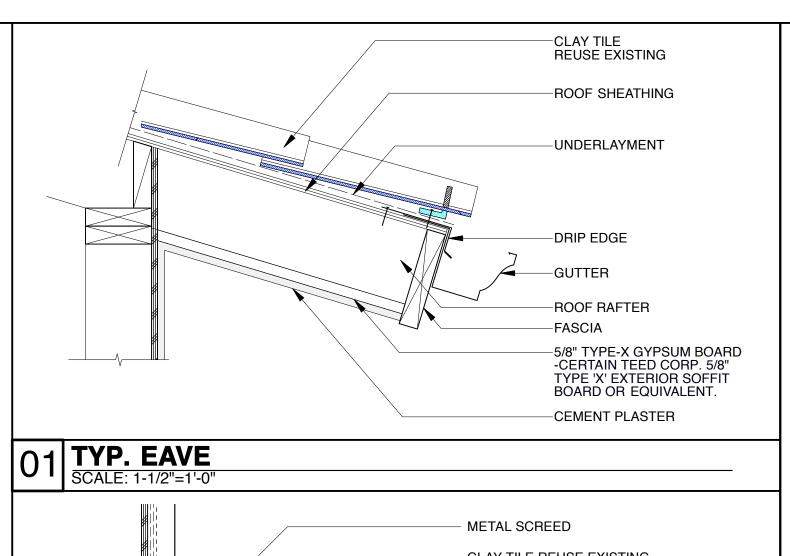


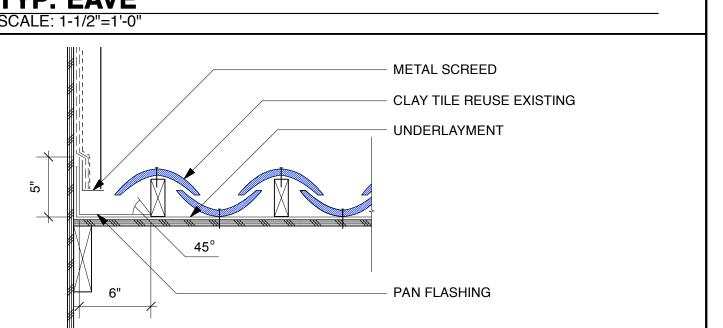
QUENTIN PENDANT SCONCE (68050993) FOR OUTDOOR USE IRON FINISH LIGHT SOURCE: 12" HIGH. 6" WIDE. 8 3/4" EXTENDS FROM WALL CA TITLE 24 COMPLIANT HIGH EFFICACY

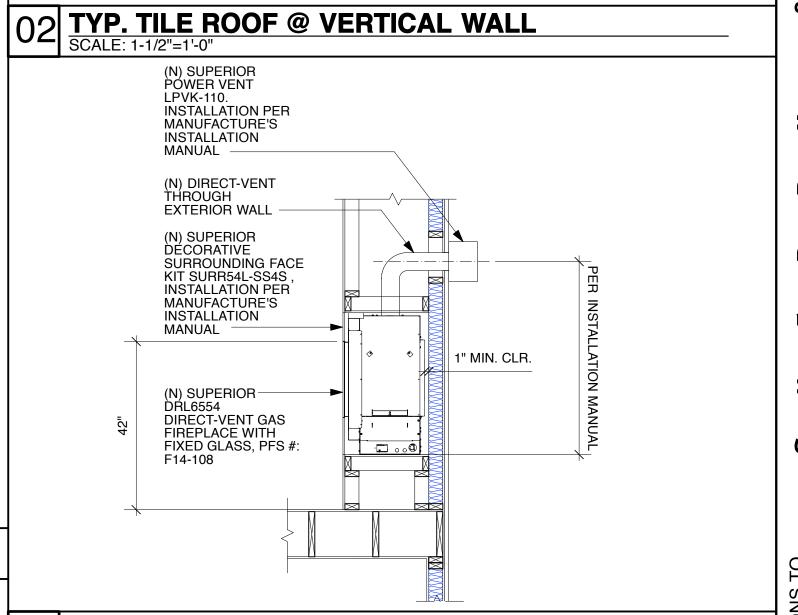
No Gap in Corner FRONT VIEW CORNER DETAIL **NOTE TO CONTRACTOR:** USE TYVEK FLASHING SYSTEM PER MANUFACTURER'S GUIDELINES. INSTALLATION GUIDELINES FOR ADDITIONAL INFORMATION NOT SHOWN USE TYVEK "FLEXWRAP" AT ALL SILLS PER MANUF. INSTALLATION REFER TO TYVEK WRB INSTALLATION GUIDELINES FOR ADDITIONAL INFORMATION NOT SHOWN HERE.

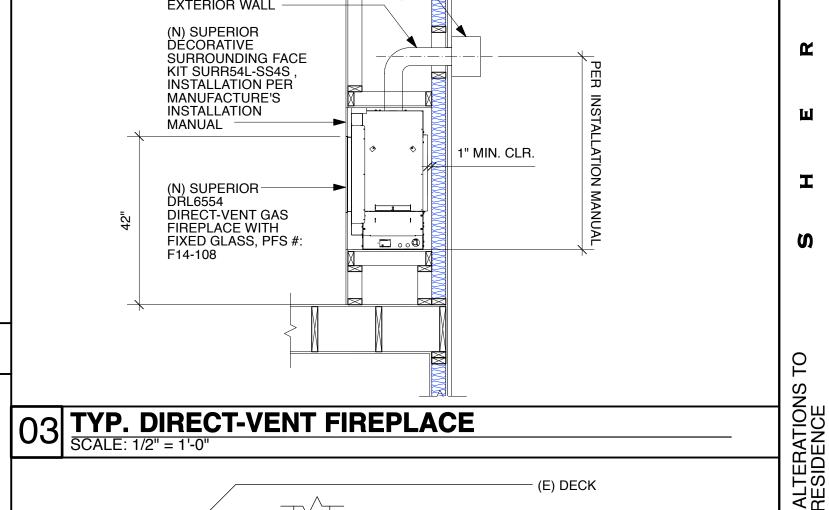
TYVEK WINDOW FLASHING SYSTEM SCALE: N.T.S.

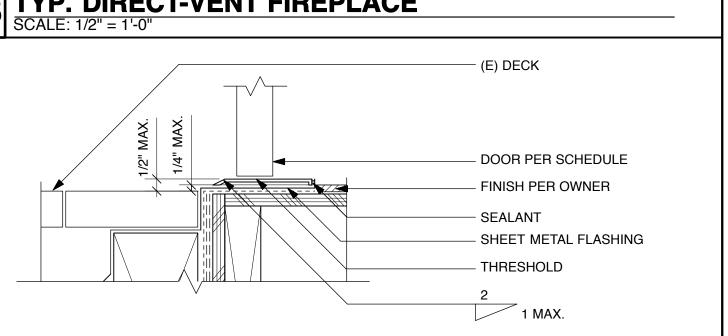


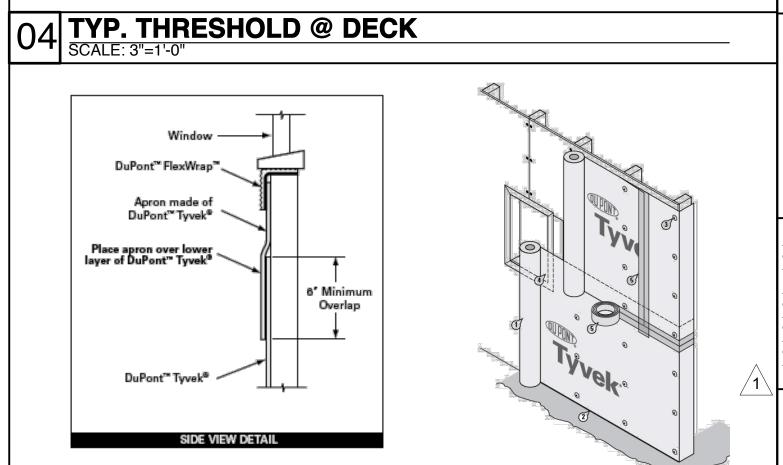












	DATE	DESCRIPTION
	09/25/18	SFDB SUBMITTAL
	10/08/18	SFDB RE-SUBMITTAL
	01/14/19	SFDB RE-SUBMITTAL
	02/21/19	BLDG DEPT SUBMITTAL
	04/17/19	BLDG DEPT RE-SUBMITTAL
1\	10/31/19	SFDB REVISION #1 SUBMITTAL

SID

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A 5.2

07 WROUGHT IRON GUARDRAIL ELEVATION
SCALE: 1-1/2" = 1'-0"

05 TYVEK WATER RESISTIVE BARRIER

10 TYP. EXTERIOR WALL LIGHT SCALE: N.T.S.

ARCHITECTURAL DETAILS

INSTALLATION YOU WILL NEED - Downspout Diverter (1) (to be purchased separately) Hacksaw - Drill with 1/4" Drill Bit and screws to secure the downspout diverter to the downspout TO START Start by choosing a location for the 1. Place the Downspout Diverter (1) on the top of the Rain Barrel and hold it up to the Downspout and mark a line with a pencil.

KoolScapes™ Environmental Rain Barrel where there is a downspout nearby.

2. Using the Hacksaw cut a straight line at the mark you just made.

3. Place the Screen Guard (2) in the open area on the top of the Rain Barrel. 4. Attach a downspout diverter to the down-

spout and connect so that it sits on the screen guard on the top of the Rain 5. Using the Level ensure your Rain Barrel is level to avoid tipping when full.

6. Ensure that the spigot (3) is inserted into the fitting in the bottom front of the

Rain Barrel – turn in clock wise direction until it is sealed tight. 7. Ensure that the drain plug (4) is also sealed tight by also turning it in clock wise.

CARE AND MAINTENANCE

The KoolScapes™ Environmental Rain Barrel needs little care.

There is an overflow spout (5) in the back to prevent water from overflowing out the top of the Rain Barrel. To divert the water away from your home simply attach a backwash hose to the overflow spout and run through your garden or lawn for

· Also if you get a lot of rainfall in your area you may want to have several rain barrels to harvest the rainwater. To connect another Rain Barrel you will need to attach tubing to the overflow spout and secure with a clamp. Connect the other end of the tubing to the overflow spout of the second rain barrel and secure with

Make sure to drain the water before winter in the areas where the temperatures drop below freezing.

This KOOLATRON product is warranted to the Cet appareil KOOLATRON est garanti à l'acheteur retail customer for 90 days from date of retail au détail pendant 90 jours à compter de sa date

the customer to Koolatron.

IMPLIED WARRANTIES

WARRANTY REGISTRATION

ing number for assistance:

locations:

all warranty work.

U.S.A.

WARRANTY AND SERVICE PROCEDURE

North America 1-800-265-8456

purchase, against defects in material and work- d'achat contre tous défauts de pièces et de fabrimanship. WHAT IS COVERED

CE QUE COUVRE LA GARANTIE • Pièces de rechange et main d'œuvre. Replacement parts and labour. • Transportation charges to customer for the • Frais d'expédition du produit réparé de Koolatron à l'adresse du client. repaired product. WHAT IS NOT COVERED CE QUE LA GARANTIE NE COUVRE PAS

warranty of merchantability are also limited to qualité marchande, se limitent également à 90

WARRANTY / GARANTIE

 Damage caused by abuse, accident, misuse, or
 Tous dégâts causés par un abus, un accident, une mauvaise utilisation ou une négligence. • Frais d'expédition de l'appareil ou de la pièce du

 Transportation of the unit or component from GARANTIES IMPLICITES Any implied warranties, including the implied Toutes garanties implicites, y compris celle de

the duration of 90 days from the date of retail jours à compter de la date d'achat. ENREGISTREMENT DE LA GARANTIE Inscrivez-vous en ligne à www.koolatron.com et

Register on-line at www.koolatron.com AND conservez le reçu de caisse daté d'origine avec ce keep the original, dated, sales receipt with this manuel. PRISE EN CHARGE DE LA GARANTIE ET PROCÉDURES DE RÉPARATION If you have a problem with your unit, or require Si vous avez un problème avec votre unité ou si replacement parts, please telephone the follow- vous avez besoin de pièces de rechange, veuillez

téléphoner au numéro suivant pour assistance : Amérique du Nord 1-800-265-8456 Koolatron has Master Service Centres at these Koolatron dispose de centres principaux de répa-

rations à ces adresses : U.S.A. 4330 Commerce Dr., Batavia, NY 14020-4102 4330 Commerce Dr., Batavia, NY 14020-4102 139 Copernicus Boulevard, Brantford, ON 139 Copernicus Boulevard, Brantford, ON A Koolatron Master Service Centre must perform Les réparations sous garantie doivent être

effectuées par un centre principal de réparations



Made in Canada/Fabriqué au Canada

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1/2018-v2

Installation Manual Guide d'installation

RBSS-50

GOCCIA

50 GALLON RAIN BARREL

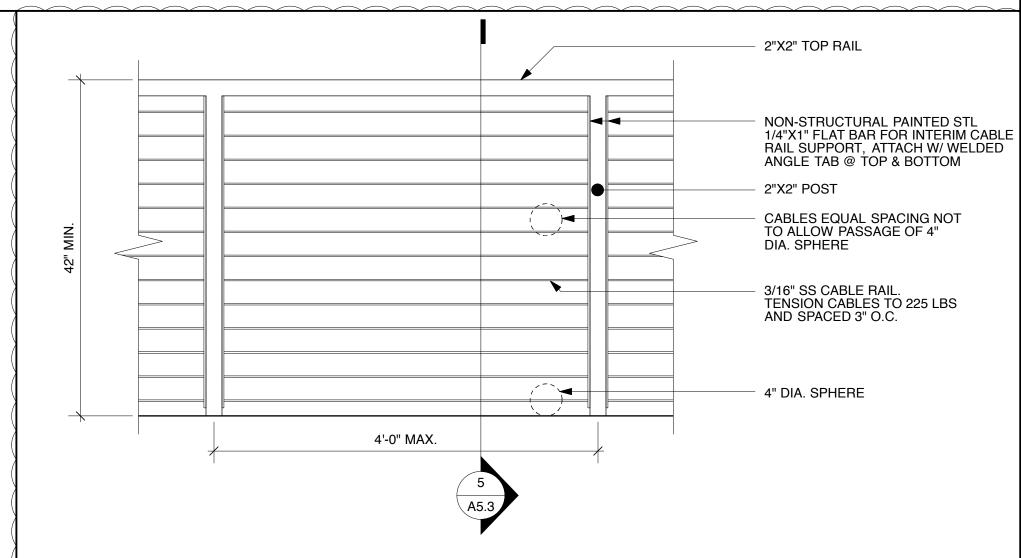
BARIL PLUVIAL À 50 GALLON

Please Read These Instructions Carefully Before Use! S.V.P., lire attentivement les instructions avant l'utilisation!

NOTES:

VERIFY WITH OWNER PRIOR TO INSTALLATION

RAIN BARREL SPECIFICATIONS (OR SIMILAR)
SCALE: N.T.S.



TYP. STAINLESS STEEL CABLE GUARDRAIL 2"X2" TOP RAIL 3/16" SS CABLE RAIL **TENSION CABLES TO 225** LBS AND SPACED 3" O.C. NON-STRUCTURAL PAINTED STL 1/4"X1" FLAT BAR FOR INTERIM CABLE RAIL SUPPORT, ATTACH W/ WELDED ANGLE TAB @ TOP & BOTTOM 2"X2" POST-

05 TYP. STAINLESS STEEL CABLE GUARDRAIL SECTION

www.goodmanmfg.com

SS-GSX14

VERIFY WITH OWNER & ARCHITECT PRIOR TO INSTALLATION

EQUIPMENT NOISE LEVELS TO NOT EXCEED 53dB AT ADJACENT PROPERTY LINE

02 AC COMPRESSOR SPECIFICATIONS SCALE: N.T.S.

GSX14

COOLING CAPACITY: 18,000 - 60,000 BTU/н

Goodman

Air Conditioning & Heating

ENERGY-EFFICIENT SPLIT SYSTEM AIR CONDITIONER 14 SEER / UP TO 12.2 EER

Contents



Standard Features **Cabinet Features**

 Energy-efficient compressor Single-speed condenser fan motor

Factory-installed filter drier

 Copper tube/aluminum fin coil Service valves with sweat connections and easy-access gauge ports

Contactor with lug connection

Ground lug connection

AHRI Certified; ETL Listed

SS-GSX14

Heavy-gauge galvanized-steel cabinet

with a louvered sound control top • Attractive Architectural Gray powder-paint

finish with 500-hour salt-spray approval Steel louver coil guard

Top and side maintenance access

 Single-panel access to controls with space provided for field-installed accessories

 When properly anchored, meets the 2010 Florida Building Code unit integrity requirements for hurricane-type winds (Anchor bracket kits available.)

To receive the 10-Year Parts Limited Warranty, online registration must be completed

www.goodmanmfg.com

PRODUCT SPECIFICATIONS

	GSX14 0181L*	GSX14 0191K*	GSX14 0241L*	GSX14 0251L*	GSX14 0301K*	GSX14 0311K*
CAPACITIES						
Nom Cool (BTU/h)	18,000	18,000	24,000	24,000	30,000	30,000
SEER/EER	14 / 12	14 / 12.2	14 / 12	14 / 12.2	14 / 12.0	14 / 12.2
Decibels	72.5	71	74.3	71	72	72
COMPRESSOR						
RLA	6.7	9.0	7.7	13.5	12.8	12.8
LRA	37.5	47.5	37.0	58.3	64	67.8
CONDENSER FAN MOTOR						
Нр	1/8	1/8	1/8	1/8	1/6	1/6
FLA	0.7	0.7	0.7	0.7	0.95	0.95
REFRIGERATION SYSTEM						
Refrigerant Line Size 1						
Liquid Line Size ("O.D.)	3/8"	3/8"	3/8"	3/8"	3/8"	3/8"
Suction Line Size ("O.D.)	3/4"	3/4"	3/4"	3/4"	3/4"	3/4"
Refrigerant Connection Size						
Liquid Valve Size ("O.D.)	3/8"	3/8"	3/8"	3/8"	3/8"	3/8"
Suction Valve Size ("O.D.) ^{2 3}	3/4"	3/4"	3/4"	3/4"	3/4"	3/4"
Valve Type	Sweat	Sweat	Sweat	Sweat	Sweat	Sweat
Charge	75	68	84	75	80	90
Included piston:	0.051	0.053	0.057	0.057	0.065	0.063
ELECTRICAL DATA						
Voltage-Phase (60 Hz)	208/230-1	208/230-1	208/230-1	208/230-1	208/230-1	208/230-1
Minimum Circuit Ampacity ⁴	9.1	12	10.3	17.6	17.0	17.0
Max. Overcurrent Protection ⁵	15 amps	20 amps	15 amps	30 amps	25 amps	25 amps
Min/Max Volts	197/253	197/253	197/253	197/253	197/253	197/253
Conduit	½" or ¾"					
EQUIPMENT WEIGHT	126	131	126	136	162	162
SHIPPING WEIGHT	141	146	141	153	180	180

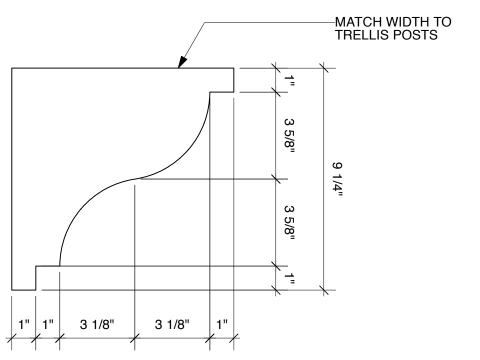
Line sizes denoted for 25' line sets, tested and rated in accordance with AHRI Standard 210/240. For other line-set lengths or sizes, refer to the installation & Operating instructions and/or the long line-set guidelines.

Installer will need to supply ¾" to ¾" adapters for suction line connections. Installer will need to supply 1/8" to 11/8" adapters for suction line connections.

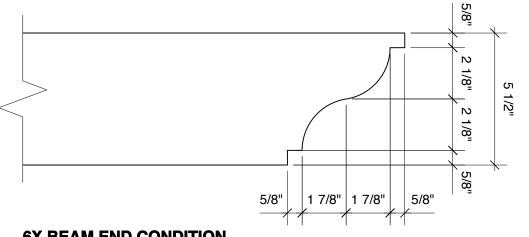
Wire size should be determined in accordance with National Electrical Codes; extensive wire runs will require larger wire sizes Must use time-delay fuses or HACR-type circuit breakers of the same size as noted.

 Always check the S&R plate for electrical data on the unit being installed. • Unit is charged with refrigerant for 15' of 1/8" liquid line. System charge must be adjusted per Installation Instructions Final Charge Procedure.

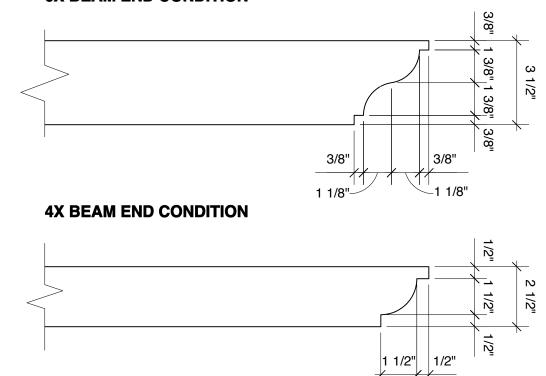
1" 3 1/8" 3 1/8" **8X BEAM END CONDITION** -MATCH WIDTH TO TRELLIS POSTS











3X BEAM END CONDITION @ FRONT ENTRY TELLIS

NOTE: ALL EXPOSED WOOD TO BE FIRE-RETARDANT-TREATED-WOOD, **REFER TO FIRE NOTES ON SHEET A 1.1**

01 TRELLIS PROFILES

SCALE: 3" = 1'-0"

DATE DESCRIPTION 09/25/18 | SFDB SUBMITTAL 10/08/18 SFDB RE-SUBMITTAL 01/14/19 | SFDB RE-SUBMITTAL 02/21/19 BLDG DEPT SUBMITTAL 04/17/19 BLDG DEPT RE-SUBMITTAL 10/31/19 SFDB REVISION #1 SUBMITTAL

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ARCHITECTURAL DETAILS

SPECIAL INSPECTIONS. **CONTRACTOR RESPONSIBILITY** AND STRUCTURAL **OBSERVATIONS**

Special inspections: The owner shall employ one or more approved agencies to perform inspections during construction on the types of work listed. These inspections are in addition to the inspections identified on this sheet.

Special inspector qualifications: The special inspector shall provide written documentation to the building official demonstrating his or her competence and relevant experience or training. Experience or training shall be considered relevant when the documented experience or training is related in complexity to the same type of special inspection activities for projects of similar complexity and material qualities. These qualifications are in addition to qualifications specified in other sections of this code.

Access for special inspection: The construction or work for which special inspection is required shall remain accessible and exposed for special inspection purposes until completion of the required special inspections. **Statement of special inspections:** The applicant shall submit a statement of special inspections.

Report requirement: The inspector(s) of record and special inspectors shall keep records of inspections. The inspector of record and special inspector shall furnish inspection reports to the building official, and to the registered design professional in responsible charge as required by the California Administrative Code. Reports shall indicate that work inspected was or was not completed in conformance to approved construction documents as required by Title 24 Parts 1 and 2. Discrepancies shall be brought to the immediate attention of the contractor for correction. If they are not corrected, the discrepancies shall be brought to the attention of the building official and to the registered design professional in responsible charge prior to the completion of that phase of the work. A final report documenting required special inspections and correction of any discrepancies noted in the inspections shall be submitted at a point in time agreed upon prior to the start of work by the applicant and the building official.

Inspection of fabricators: Where fabrication of structural load-bearing members and assemblies is being performed on the premises of a fabricator's shop, special inspection of the fabricated items shall be required by this section and as required elsewhere in this code.

Fabrication and implementation procedures: The special inspector shall verify that the fabricator maintains detailed fabrication and quality control procedures that provide a basis for inspection control of the workmanship and the fabricator's ability to conform to approved construction documents and referenced standards. The special inspector shall review the procedures for completeness and adequacy relative to the code requirements for the fabricator's scope of

Content of statement of special inspections:

- The statement of special inspections shall identify the following: 1. The materials, systems, components and work required to have special inspection or testing by the building official or by the registered design professional responsible for each portion of the work.
- The type and extent of each special inspection. The type and extent of each test.
- seismic or wind resistance as specified in Sections 1705A.10,
- 1705A.11 and 1705A.12. 5. For each type of special inspection, identification as to whether it will be continuous special inspection or periodic special inspection.

Seismic requirements in the statement of special inspections: Where Section 1705A.11 or

1705A.12 specifies special inspection is required, testing or certification for seismic resistance, the statement of special inspections shall identify the equipment/components that require special seismic certification and seismic force-resisting systems that are subject to special inspections.

Inspection of structural welding: Inspection of all shop and field welding operations shall be made by a qualified welding inspector approved by the enforcement agency. The minimum requirements for a qualified welding inspector shall be as those for an AWS certified welding inspector (CWI), as defined in the provisions of the AWS QC1. All welding inspectors shall be as approved by the enforcement agency.

The welding inspector shall make a systematic daily record of all welds. In addition to other required records, this record shall include: 1. Identification marks of welders.

2. List of defective welds. 3. Manner of correction of defects. The welding inspector shall check the material, details of construction and procedure, as well as workmanship of the welds. The inspector shall verify that the

installation of end-welded stud shear connectors is in accordance with the requirements of AWS D1.1 and the approved plans and specifications. The inspector shall furnish the architect, structural engineer, and the enforcement agency with a verified report that the welding is proper and has been done in conformity with AWS D1.1, D1.8, and the approved construction documents.

Concrete construction: The special inspections and verifications for concrete construction shall be as required by this section and Table 1705A.3.

Exception: Special inspections shall not be required for concrete patios, driveways and sidewalks, on grade. **Materials:** In the absence of sufficient data or documentation providing evidence of conformance to quality standards for materials in Chapter 3 of ACI 318, the building official shall require testing of materials in accordance with the appropriate standards and criteria for the material in Chapter 3 of ACI 318. Weldability of reinforcemen except that which conforms to ASTM A 706, shall be determined in accordance with the requirements of Section 3.5.2 of ACI 318.

Batch plant inspection: Except as provided under Section 1705A.4.3, the quality and quantity of materials used in transit-mixed concrete and in batched aggregates shall be continuously inspected by an approved special inspector at the location where materials are measured.

Waiver of continuous batch plant inspection:

Continuous batch plant inspection may be waived by the registered design professional, subject to approval by the enforcement agency under either of the following conditions 1. The concrete plant complies fully with the requirements of ASTM C 94, Sections 8 and 9, and has a

current certificate from the National Ready Mixed Concrete Association or another agency acceptable to the enforcement agency. The certification shall indicate that the plant has automatic batching and recording capabilities.

2. For single -story light-framed - buildings and isolated foundations supporting equipment only, where the specified compressive strength f'c of the concrete delivered to the jobsite is 3,500 psi (24.13 MPa) and where the f 'c used in design is not greater than 3,000 psi (20.68 MPa). When continuous batch plant inspection is waived, the following periodic inspection requirements shall apply and shall be described in the construction

1. Qualified technician of the testing laboratory shall check the first batch at the start of the

2. Licensed weighmaster to positively identify materials as to quantity and certify to each load by a batch ticket.

3. Batch tickets, including material quantities and weights shall accompany the load, shall be transmitted to the inspector of record by a truck driver with load identified thereon. The load shall not be placed without a batch ticket identifying the mix. The inspector will keep adaily record of placements, identifying each truck, its load, and time of receipt, and approximate location of deposit in the structure and will transmit a copy of the daily record to the enforcement agency.

Masonry construction: Masonry construction shallbe inspected and verified in accordance with TMS 402/ACI530/ASCE 5 and TMS 602/ACI 530.1/ASCE 6 quality assur-ance program, as set forth in Table 1.19.3, Level C require-ments. Inspection and testing of post-installed anchors inmasonry shall be required in accordance with requirements for concrete in Chapters 17A and 19A

Wood construction: Special inspections of the fabrication process of prefabricated wood structural elements and assemblies shall be in accordance with Section 1704A.2.5.

Manufactured open web trusses: The manufacture of open web trusses shall be continuously inspected by a qualified special inspector approved by the enforcement agency.

The special inspector shall verify that proper quality control procedures and tests have been employed for all materials and the manufacturing process, and shall perform visual inspection of the finished product. Each inspected truss shall be stamped with an identification mark by the special inspector.

Soils: Special inspections for existing site soil conditions, fill placement and load-bearing requirements shall be as required by this section and Table 1705A.6. The approved geotechnical report, and the construction documents prepared by the registered design professionals shall be used to determine compliance. During fill placement, the special inspector shall determine that proper materials and procedures are used in accordance with the provisions of the approved geotechnical

Exception: Where Section 1803A does not require reporting of materials and procedures for fill placement, the special inspector shall verify that the in-place dry density of the compacted fill is not less than 90 percent of the maximum

dry density at optimum moisture content determined in accordance with ASTM D 1557. **Soil fill:** All fills used to support the foundations of any building or structure shall be continuously inspected by the geotechnical engineer or his or her qualified representative. It shall be the responsibility of the geotechnical engineer to verify that fills meet the requirements

of the approved construction documents and to coordinate all fill inspection and testing during the construction involving such fills. The duties of the geotechnical engineer or his or her qualified representative shall include, but need not be limited to, theinspection of cleared areas and benches prepared

to receive fill; inspection of the removal of all unsuitable soils and other materials; the approval of soils to be used as fill material; the inspection of placement and compaction of fill materials; the testing of the completed fills; the inspection or review of geotechnical drainage devices, buttress fills or other similar protective measures in accordance with the approved construction documents.

A verified report shall be submitted by the geotechnical engineer as required by the California Administrative Code. The report shall indicate that all tests and inspection required by the approved construction documents were completed and that the tested materials and/or inspected work meet the requirements of the approved construction documents.

Cast-in-place deep foundations: Special inspections shall be performed during installation and testing of cast-in-place deep foundation elements as required by Table 1705.8. The approved geotechnical report, and the construction documents prepared by the registered design professionals, shall be used to determine compliance.

Helical pile foundations: Special inspections shall be performed continuously during installation of helical pile foundations. The information recorded shall include installation equipment used, pile dimensions, tip elevations, final depth, final installation torque and other pertinent installation data as required by the registered design professional in responsible charge. The approved geotechnical report and the construction documents prepared by the registered design professional shall be used to determine compliance.

Structural wood: Continuous special inspection is required during field gluing operations of elements of the main windforce-resisting system. Periodic special inspection is required for nailing, bolting, anchoring and other fastening of components within the main windforceresisting system, including wood shear walls, wood diaphragms, drag struts, braces and hold-downs.

Cold-formed steel light-frame construction: Periodic special inspection is required during welding operations of elements of the main windforce-resisting system. Periodic special inspection is required for screw attachment, bolting, anchoring and other fastening of components within the main windforce-resisting system, including shear walls, braces, diaphragms, collectors (drag struts) and hold-downs.

STEEL CONSTRUCTION

704.3 Steel construction. The special inspections for steel elements of buildings and structures shall be as required by Section 1704.3 and Table 1704.3.

Special inspection of the steel fabrication process shall not be required where the fabricator does not perform any welding thermal cutting or heating operation of any kind as part of the fabrication process. In such cases, the fabricator shall be required to submit a detailed procedure for material control that demonstrates the fabricator's ability to maintain suitable records and procedures such that at anytime during the fabrication process, the material specification, grade and mill test reports for main stress-carrying elements are capable of being determined.

The special inspector need not be continuously present during welding of the following items. provided the materials, welding procedures and qualifications of welders are verified prior to the start of work; periodic inspections are made of the work in progress and a visual inspection of all welds is made prior to completion or prior to shipment of shop welding.

2.1. Single pass fillet welds not exceeding $\frac{5}{16}$ inch (7.9MM) in size 2. Floor and roof deck welding. 2.3. Welded studs when used for structural diaphragm. 2.4. Welded sheet steel for cold-formed steel members. 2.5. Welding of stairs and railing systems.

1704.3.1 Welding. Welding inspection and welding inspection qualification shall be in accordance with this section.

> 1704.3.1.1 Structural steel. Welding inspection and welding inspector qualification for structural steel shell be in accordance with AWS D1.1.

Stone Veneer.

anchored directly to masonry, concrete of to stud construction by one of the following methods. 1. With concrete and masonry backing, anchor ties shall be not less than 0.1055-inch (2.68 MM) corrosion-resistant wire, or approved equal, formed beyond the base of the backing. The legs of the loops shall be not less than 6 inches (152MM) in length bent at right angles and laid in the mortar joint, and spaced so that the eyes or loops are 12 inches (305 MM) max. on center (O.C.) in both directions. There shall be provided not less than a 0.1055-inch (2.68

Stone veneer units not exceeding 10 inches (254 MM) in thickness shall be

MM) corrosion-resistant wire tie, or approved equal, threaded through the exposed loops for every 2 square feet (0.2M2) of stone veneer. This tie shall be a loop having legs not less than 15 inches (381 MM) in length bent so that it will lie in the stone veneer mortar ioint. The last 2 inches (51MM) of each wire leg shall have a right-angle bend. one-inch (25MM) minimum thickness of cement grout shall be placed between the backing and the stone veneer.

2. With stud backing, a 2-inch x 2-inch (51mmx51mm) 0.0625-INCH (1.59 mm) corrosion-resistant wire mesh with two layers of water-restrictive barrier in accordance with Section 1403.3 shall be applied directly to wood studs spaced a max. of 16 inches (406 MM) o.c. On studs, the mesh shall be attached with 2-inch-long (51mm) corrosion-resistante steel wire furring nails at 4 inches (102 mm) o.c. providing a minimum 1.125-inch (29mm) penetration into each stud and with 8d common nails at 8 inches (203mm) o.c. into top and bottom plates or with equivalent wire ties. There shall be not less than a 0.1055 inch (2.68 mm) corrosion-resistant wire, or approved equal, looped through the mesh every 2 square feet (0.2 m2) of stone veneer. This tie shall be a loop having legs not less than 15 inches (381mm) in length, so bent that it will lie in the stone veneer mortar joint. The last 2 inches (51 mm) of each wire leg shall have a right-angle bend. One inch (25mm) minimum thickness of cement grout shall be placed between the backing and the stone veneer.

INTERIOR MOISTURE CONTROL

Concrete stab to Building Code, CCR, Title 24, Part 2, Chapter 19.

Capillary break. A capillary break shall be installed in compliance with at least one of the following A 4-inch (101.6mm) thick base of ½ inch (12.7mm) or larger clean

aggregate shall be provided with a vapor barrier in direct contact with concrete and a concrete mix design, which will address bleeding, shrinkage, and curling, shall be used. Moisture content building materials. Building materials with visible signs of water damage shall not be installed. Wall and floor framing shall

not be enclosed when the framing members exceed 19 percent moisture content. Moisture content shall be verified in compliance with the following:

Moisture content shall be determined with either a probe-type or content-type moisture meter.

Moisture readings shall be taken at a point 2 feet (610 mm) to 4 feet (1219 mm) from the grade stamped end of each piece to be verified. At least three random moisture readings shall be performed on wall and floor framing with documentation acceptable to the enforcing agency provided at the time of approval to enclose the wall and floor framing.

Insulation products which are visibly wet of have a high moisture content shall be replaced or allowed to dry prior to enclosure in wall or floor cavities. Wet-applied insulation products shall follow the manufacturers' drying recommendations prior to enclosure.

CONCRETE BLOCK

CONCRETE BLOCK SHALL BE MANUFACTURED FROM LIGHTWEIGHT AGGREGATE AND SHALL CONFORM TO GRADE "N" HOLLOW LOAD BEARING UNITS ASTM C-90.

MORTAR SHALL BE UBC TYPE "S" AND SHALL HAVE A MINIMUM STRENGTH OF 2000 PSI AT 28

GROUT SHALL BE TRANSIT MIXED AND CONTAIN 7 SACKS OF CEMENT PER CUBIC YARD OF ROUT. MAXIMUM SLUMP SHALL BE 9-11". MINIMUM ULTIMATE COMPRESSIVE STRENGTH SHALL BE 2000 PSI AT 28 DAYS.

4. OPEN END BLOCK SHALL BE USED THROUGHOUT.

GROUT SHALL BE POURED IN 4' LIFTS.

6. SPLICES IN REINFORCING BARS SHALL BE A

MINIMUM OF 48 BAR DIAMETER.

7. ALL WALLS SHALL BE GROUTED SOLID. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IS #4 @ 16" O.C. VERTICAL AND #4 @

24" O.C. HORIZONTAL WITH (2) #3 CONTINUOUS @

THE TOP OF THE WALL. **CONCRETE MASONRY UNITS:** Concrete masonry units shall conform to the following standards: ASTM C 55 for concrete brick; ASTM C 73 for calcium silicate face brick; ASTM C 90 for load-bearing concrete masonry units or ASTM C 744 for prefaced concrete and calcium silicate

10. MORTAR: Mortar for use in masonry construction shall conform to ASTM C 270 and Articles 2.1 and 2.6 A of TMS 602/ACI 530.1/ASCE 6, except for mortars listed in Section 2103A.10. Type S mortar conforming to ASTM C 270 shall be used for glass unit masonry

STRUCTURAL STEEL

ALL STRUCTURAL STEEL SHALL COMPLY WITH; American Institute of Steel Construction (AISC) Specification for Structural Steel Buildings, ANSI/AISC 360-10 Prequalified Connections for Special and Intermediate Steel Moment Frames for Seismic Applications, ANSI/AISC 385-10

American Society of Nondestructive Testing (ASNT) Recommended Practice for the Training and Testing of Nondestructive Testing Personnel, ASNT SNT TC-1a-2001

Standard for the Qualification and Certification of Nondestructive Testing Personnel, ANSI/ASNT CP-189-2001

American Welding Society (AWS)

Structural Welding Code-Steel AWS D1.1/D1.1 M-2004 Specification for structural joints using ASTM A325 joints or A490 Bolts, 2004 Research Coucil on Structural connections (RCSC)

Standard Methods for Determination of the Diffusible Hydrogen Content of Martensitic, Bainitic, and Ferritic Steel Weld Metal Produced by Arc Welding, AWS A4.3-93R Standard Methods for Mechanical Testing of Welds - U.S.

Customary, ANSI/ AWS B4.0-98 Standard Methods for Mechanical Testing of Welds - Metric Only, ANSI/AWS

B4.0M:2000 Standard for the Qualification of Welding Inspectors, AWS Oxygen Cutting Surface Roughness Gauge and Wall Chart for Criteria Describing Oxygen - Cut Surfaces, AWS C4.1 Federal Emergency Management Agency (FEMA) Recommended Seismic Design Criteria for New Steel Moment-Frame Buildings, FEMA 350, July 2000

ALL STRUCTURAL STEEL SHALL INCLUDE THE FOLLOWING: SHOP DRAWINGS: Shop drawings shall include items required by the Specification and the following, as applicable:

(1) Desidnation of the members and connections that are part of the

(2) Connection Material specifications

(3) Locations of demand critical shop welds (4) Locations and dimensions of protected zones (5) Gusset plates drawn to scale when they are detailed to accommodate

inelastic rotation. (6) Welding requirements as specified in Appendix W, Section

ERECTION DRAWINGS: Erection Drawings shall include items required by the Specification and the following, as applicable:

(1) Designation of the members and connections that are part of the

(2) Field connection material specifications and sizes (3) Locations of demand critical field welds

4) Locations and dimensions of protected zone

(5) Locations of pretensioned bolts. (6) Field welding requirements as specified in Appendix W, Section

ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A-36 UNLESS OTHERWISE SHOWN. MINIMUM YIELD STRENGTH SHALL BE 36,000 PSI.

PIPE COLUMNS SHALL CONFORM TO ASTM, MINIMUM YIELD 46,000 PSL ALL STEEL TUBE SHALL CONFORM TO ASTM A-500. MINIMUM YIELD 46,000 PSI.

ALL STEEL FABRICATION TO BE DONE IN A SHOP APPROVED BY THE BUILDING DEPARTMENT.

STRUCTURAL STEEL FABRICATOR TO FURNISH SHOP DRAWINGS OF ALL STEEL FOR ENGINEERS APPROVAL BEFORE FABRICATION. NO FIELD MODIFICATIONS ALLOWED UNLESS APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.

ALL WELDING SHALL BE PERFORMED BY WELDERS HAVING CURRENT WELDING CERTIFICATES.

FIELD WELDING IS DESIGNED FOR FULL STRESS. CONTINUOUS INSPECTION IS REQUIRED.

ALL BOLTS SHALL BE UNFINISHED BOLTS CONFORMING TO ASTM A-307 OF THE SIZE SHOWN ON THE DRAWINGS UNLESS OTHERWISE SHOWN OR

10. UNLESS OTHERWISE NOTED OF SHOWN, ALL BEAM CONNECTIONS SHALL BE STANDARD FRAMED BEAM CONNECTIONS WITH 3/4" UNFINISHED BOLTS IN ACCORDANCE WITH AISC RECOMMENDATIONS. MODIFY ANGLE SIZES TO FIT SPECIAL CONDITIONS.

11. USE AISC USUAL GAUGES FOR BOLTS IN ALL STEEL SECTIONS, MODIFIED AS REQUIRED BY CONDITIONS UNLESS OTHERWISE SHOWN OR NOTED.

12. ALL STEEL COLUMNS SHALL BE MILLED EACH END TO FIT FLUSH WITH BASE PLATE CAP OR END TO END.

13. ALL ANCHOR BOLT INSTALLATION SHALL REQUIRE SPECIAL INSPECTION.

TESTING AND FIELD VERIFICATION: TESTS OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS. HIGH-STRENGTH BOLTS, NUTS AND WASHERS SHALL BE SAMPLED AND TESTED BY AN APPROVED INDEPENDENT TESTING LABORATORY FOR CONFORMANCE WITH THE REQUIREMENTS OF APPLICABLE

ASTM STANDARDS. [OSHPD 1 AND 4] A MINIMUM OF THREE SAMPLES PER LOT, DEFINED IN THE ASTM STANDARDS FOR BOLTS [AND NOT NUTS AND WASHERS], SHALL BE TESTED FOR TENSILE PROPERTIES,

EXCEED THREE SAMPLES PER 400 BOLTS.

BUT NEED NOT

2213A.2 TESTS OF END-WELDED STUDS. END-WELDED STUDS BE TESTED PER THE REQUIREMENTS OF THE AWS D1.1 **SECTIONS 7.7** AND 7.8.

CONCRETE

ALL CONCRETE FOR THE FOOTINGS, FLATWORK AND MISCELLANEOUS ITEMS SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE ON THE DRAWINGS.

REINFORCING BARS SHALL BE OF INTERMEDIATE GRADE CONFORMING TO ASTM A 615, GRADE 40 FOR #2 AND #3 BARS AND GRADE 60 FOR #4 BARS AND LARGER.

3. CEMENT SHALL BE TYPE II, LOW ALKALI, CONFORMING TO ASTM C-150.

ALL AGGREGATE USED IN CONCRETE SHALL CONFORM TO ASTM C-33. MAXIMUM AGGREGATE SIZE SHALL BE 1".

SPLICES OF REINFORCING STEEL SHALL BE -CLASS B IN ACCORDANCE W/12.15.1 ACI 318, LAPPED A MINIMUM OF 48 DIAMETERS AND SECURELY WIRED TOGETHER. SPLICES OF ADJACENT REINFORCING BARS SHALL BE STAGGERED WHEREVER POSSIBLE. SEE DRAWINGS FOR PARTICULAR REQUIREMENTS FOR SPLICE BREAKS. ALL BARS ARE TO BE CONTINUOUS AROUND CORNERS AND INTERSECTIONS.

MINIMUM CONCRETE COVER FOR REINFORCING

OTHER PER UNIFORM BUILDING CODE.

SHALL BE: CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH....3" CAST IN FORMS AND EXPOSED TO EARTH OR WEATHER....2"

7. LOCATIONS OF SLEEVES FOR PIPES, AND FOR PIPES INTENDED TO BE CAST IN CONCRETE, FOR WHICH NO SPECIFIC DETAILS ARE SHOWN SHALL BE SUBJECT TO THE REVIEW OF THE STRUCTURAL

EXTERIOR CONCRETE SLABS ON GRADE SHALL BE REINFORCED WITH #3'S @ 18" O.C. EACH WAY, PLACED 2" FROM THE TOP OF THE SLAB, UNLESS OTHERWISE NOTED ON THE PLANS. THIS REINFORCEMENT SHALL BE SUPPORTED BY CONCRETE BLOCKS 2" THICK, SPACED AT

ENGINEER.

9. ALL ANCHOR BOLTS, REINFORCING STEEL, DOWELS, INSERTS, ETC., SHALL BE WELL SECURED

IN POSITION PRIOR TO POURING CONCRETE. 10. CONCRETE SHALL CONTAIN A MINIMUM OF 5 SACKS OF CEMENT PER CUBIC YARD AND SHALL HAVE A SLUMP OF NO GREATER THAN 4". ALL

CONCRETE SHALL BE VIBRATED IMMEDIATELY PREFABRICATED TRUSSES

INTERVALS NOT EXCEEDING 4'-0".

Truss design drawings. The written. graphic and pictorial depiction of each individual truss shall be provided to the building official for approval prior to installation. Truss design drawings shall also be provided with the shipment of trusses delivered to the job site. Truss design drawings shall include, at a minimum, the information specified below:

1. Slope or depth, span and spacing; 2. Location of all joints and support locations; 3. Number of plies if greater than one; 4. Required bearing widths: 5. Design loads as applicable, including;

5.1. Top chord live load; 5.2. Top chord dead load 5.3. Bottom chord live load; 5.4. Bottom chord dead load; 5.5. Additional loads and locations; and 5.6. Environmental design criteria and loads

(wind, rain, snow, seismic, etc.). 6. Other lateral loads, including drag strut loads; 7. Adjustments to wood member and metal connector plate design value for conditions of use; 8. Maximum reaction force and direction, including maximum uplift reaction forces where applicable; 9. Metal-connector-plate type, size and thickness or gage, and the dimensioned location of each metal

connector plate except where symmetrically located relative to the joint interface; 10. Size, species and grade for each wood member; 11. Truss-to-truss connections and truss field assembly requirements;

12. Calculated span-to-deflection ratio and maximum vertical and horizontal deflection for live and total load as applicable; 13. Maximum axial tension and compression forces in the truss members; and 14. Required permanent individual truss member restraint location and the method and details of

restraint/bracing to be used in accordance with

Section 2303.4.1.2.

COMPLETE CALCULATIONS SHOWING INTERNAL LAYOUT, MEMBER FORCES AND STRESS CONTROL POINTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT FOR APPROVAL AND TO THE STRUCTURAL ENGINEER FOR REVIEW. ALL CALCULATIONS SHALL BE SIGNED BY A PROFESSIONAL CIVIL OR STRUCTURAL ENGINEER

REGISTERED IN THE STATE OF CALIFORNIA.

FABRICATOR SHALL FURNISH NAME, ADDRESS, AND PHONE NUMBER OF THE AGENCY INSPECTING FABRICATION OPERATION TO THE COUNTY BUILDING OFFICIAL AND THE ARCHITECT.

4. TO CHORDS SHALL BE DESIGNED TO ACCOMMODATE CLOSELY SPACED PLYWOOD NAILING WHERE SHOWN ON PLANS

5. TRUSSES SHALL BE DESIGNED FOR 12.5 PSF DEAD + 20 PSF LIVE LOAD AT ROOF AREAS. LIVE LOADS MAY BE REDUCED FOR LOAD DURATION AND/OR TRIBUTARY AREA. LIVE LOAD DEFLECTIONS SHALL BE LIMITED TO L/360. DEAD PLUS LIVE LOAD DEFLECTIONS SHALL BE LIMITED TO L/240.

TIMBER

FRAMING AND SHEATHING GRADES SHALL BE AS FOLLOWS:

ROOF RAFTERS.....DOUG FIR NO. 2 FLOOR JOISTSDOUG FIR NO. 1 4X, 6X, & 8X BEAMS/HEADERS ... DOUG FIR NO. 1 WALL STUDS......DOUG FIR CONSTRUCTION BLOCKING, STRIPPING, & MISC...DOUG FIR NO. 3 PLYWOOD.....CD-X STRUCT I

2. MINIMUM NAILING PER UNIFORM BUILDING CODE, SEE TYPICAL NAILING SCHEDULE ON PLAN.

3. ALL NAILS SHALL BE COMMON SIZE. NO BOX NAILS OR SINKERS ALLOWED.

ANCHORAGE OF NON-BEARING STUD WALLS ON CONCRETE SHALL BE ¾" DIAMETER X 6" ANCHOR BOLTS AT 4'-0" O.C. OR POWDER DRIVEN PINS WHICH SHALL PENETRATE INTO CONCRETE A MINIMUM OF 1½". POWDER DRIVEN ANCHORS SHALL BE SPACED AT 32" O.C.

5. EXCEPT WHERE REQ'D. AT SHEAR WALLS, ANCHOR BOLTS SHALL BE %" MINIMUM DIAMETER, SPACED 6' MAXIMUM ON CENTER, WITH 7" MINIMUM EMBEDMENT, 12" MAXIMUM FROM CORNERS WITH A MINIMUM 3"x3"x.229" SQUARE WASHER @ EACH ANCHOR BOLT. UBC 1806.6, 1806.6.1. THIS IS A TYPICAL REQUIREMENT FOR ALL BEARING AND EXTERIOR WALLS, U.N.O.

HOLES IN WOOD FOR BOLTS SHALL BE DRILLED 1/16" DIAMETER LARGER THAN THE NOMINAL SIZE OF THE BOLT UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTS SHALL HAVE STANDARD CUT WASHERS UNDER HEADS AND/OR NUTS WHERE IN CONTACT

WHERE STUD WALL TERMINATES AT A CONCRETE OR MASONRY WALL, FASTEN THE LAST STUD TO THE WALL WITH 3/8" DIAMETER X 6" LONG LAG BOLTS AT THE TOP, BOTTOM, AND MID-HEIGHT OF THE STUD. MAXIMUM VERTICAL SPACING OF ANCHORS SHALL BE 6'-0".

9. LAG BOLTS SHALL BE PRE-DRILLED AS RECOMMENDED BY UBC STANDARDS AND SCREWED INTO PLACE.

10. SPLICES IN UPPER AND LOWER PLATES IN THE TOP OF STUD WALLS SHALL BE STAGGERED AT LEAST 4'-0". DOUBLE TOP PLATE SPLICES ARE TO HAVE (12) 16d NAILS EACH SIDE OF SPLICE. ANY DISCONTINUITY IN PLATE IS TO BE CONNECTED WITH A FULLY NAILED "SIMPSON" ST6236 STRAP, OR EQUAL.

11. ALL 2X JOISTS AND RAFTERS SHALL BE SOLID BLOCKED AT POINTS OF BEARING. WOOD CROSS-BRIDGING NOT LESS THAN 2" BY 3" NOMINAL, METAL CROSS-BRIDGING OF EQUAL STRENGTH, OR SOLID BLOCKING, SHALL BE PLACED BETWEEN JOISTS WHERE THE JOIST OR RAFTER SPAN EXCEEDS EIGHT (8) FEET. CROSS-BRIDGING OR BLOCKING MAY BE OMITTED FOR ROOF AND CEILING JOISTS 8" AND LESS IN DEPTH, UNLESS OTHERWISE NOTED ON PLANS.

12. NO NOTCHING OR CUTTING OF FRAMING MEMBERS IS PERMITTED WITHOUT WRITTEN APPROVAL OF ENGINEER.

AT ALL UNBLOCKED EDGES OF PLYWOOD SHEATHING. 14. MINIMUM DIMENSION OF ANY PLYWOOD SHEET

13. PROVIDE ONE PLYWOOD CLIP BETWEEN EACH JOIST

SHALL BE 24" AND THE MINIMUM AREA SHALL BE 8 SQUARE FEET. 15. MACHINE APPLIED NAILING: 1) SATISFACTORY INSTALLATION SHALL BE DEMONSTRATED ON THE JOB. 2) THE STRUCTURAL ENGINEER HAS THE RIGHT

SATISFACTORY PERFORMANCE. 16. ALL TIMBER CONNECTORS SHALL BE GALVANIZED OR PAINTED WITH CORROSION RESISTANT POLYMER

TO REVIEW APPLICATION TO CONFIRM CONTINUED

17. ALL SHEET METAL FRAMING CONNECTORS SHOWN ON THE PLANS SHALL BE "SIMPSON" STRONG-TIE, BY SIMPSON CO., OR EQUAL. UNLESS NOTED OTHERWISE ON PLANS, INSTALL CONNECTORS WITH THE SIZE AND NUMBER OF BOLTS AS RECOMMENDED BY THE

MANUFACTURER IN THE LATEST CATALOG. 18. MEMBERS EXPOSED TO VIEW: 1) SELECT FOR BEST APPEARANCE AVAILABLE IN GRADE SPECIFIED, FREE OF HEART RINGS, CHECKS, AND SPLITS. 2) GRADE STAMPS EXPOSED TO VIEW WILL NOT BE ACCEPTABLE. 3) REMOVE ALL STAINS OR GOUGES

19. SILL PLATES RESTING ON CONCRETE OR MASONRY SHALL BE DOUGLAS FIR PRESSURE IMPREGNATED WITH AN APPROVED PRESERVATIVE

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Santa Barbara, CA 93101

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PRIOR TO INSTALLATION.

GENERAL NOTES

ALL MATERIALS AND WORKMANSHIP SHALL BE SUBJECT TO THE REVIEW OF THE OWNER AND STRUCTURAL ENGINEER.

APPARENT DISCREPANCIES OR AMBIGUITIES ON THE DRAWINGS AND/OR SPECIFICATIONS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER IN WRITING BEFORE PROCEEDING WITH WORK.

ALL WORK DONE UNDER THIS CONTRACT SHALL BE IN COMPLIANCE WITH THE 2016 EDITION OF THE CALIFORNIA BUILDING CODE.

IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO DESIGN AND INSTALL ALL TEMPORARY BRACING AND SHORING TO INSURE SAFETY UNTIL THE WORK IS COMPLETED.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING WORK. THE OWNER AND STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES. THE CONTRACTOR IS RESPONSIBLE FOR CHECKING AND COORDINATING ALL DIMENSIONS. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND NON-STRUCTURAL ITEMS NOT SHOWN ON STRUCTURAL PLANS.

ALL SCAFFOLDING AND SHORING SHALL COMPLY WITH THE RULES AND REGULATIONS OF THE INDUSTRIAL SAFETY COMMISSION OF THE STATE OF CALIFORNIA.

THE CONTRACTOR IS REQUIRED TO ASSUME SOLE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING CONSTRUCTION OF THE PROJECT. INCLUDING SAFETY OF ALL PERSONS AND PROPERTY: THAT THIS REQUIREMENT MADE TO APPLIED CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS, AND THE CONTRACTOR FURTHER AGREES TO DEFEND, INDEMNIFY AND HOLD HARMLESS THE STRUCTURAL ENGINEER FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING LIABILITY ARISING FROM THE SLOE NEGLIGENCE OF THE STRUCTURAL ENGINEER.

ALL WATERPROOFING, DRAINAGE, SEALING, OR ANY AND ALL WATER AND/OR MOISTURE RELATED ISSUES ARE THE FULL RESPONSIBILITY OF THE CONTRACTOR.

ANY DISCREPANCIES BETWEEN THIS DRAWING AND ANY OTHER DRAWING AND/OR EXISTING OR NEW CONDITION SHALL BE REPORTED TO THE ENGINEER PRIOR TO PROCEEDING WITH WORK AND SHALL BE THE CONTRACTORS RESPONSIBILITY.

IF ANY DISCREPANCIES EXIST BETWEEN THESE DRAWINGS AND ACTUAL CONDITIONS AND/OR ARCHITECTURAL DRAWINGS; CONTRACTOR IS TO NOTIFY ENGINEER IN WRITING PRIOR TO PROCEEDING WITH WORK.

NOTE: THE INTENT OF STRUCTURAL DESIGN CONTAINED IN THESE DRAWINGS IS. ALTERATIONS. BUILDING IS BEING REVISED LATERALLY, SHEAR LINES ARE BEING STRENGTHEDNED, THE BUILDING IS NOT BEING UPGRADED FULLY TO CURRENT CODE. THE ENGINEER IS NOT RESPONSIBLE FOR FULL CURRENT CODE COMPLIANCE. NO SIGNFICANT FOUNDATION WORK IS BEING PERFORMED.

FOUNDATIONS

1. FOOTING DEPTHS ARE TO BE MIN. 24" INTO FIRM NATURAL GRADE. A SOILS ENGINEER IS TO BE NOTIFIED AND REVIEW AND APPROVE ALL BOTTOM OF FOOTING EXCAVATIONS PRIOR TO POUR. CONTRACTOR RESPONSIBLE TO OBTAIN RESPONSE FROM SOILS

ENGINEER. 2. PRIOR TO POURING CONCRETE FOUNDATIONS, ALL LOOSE EARTH, WATER, AND DEBRIS SHALL BE

REMOVED FROM THE FOUNDATION BED. 3. DESIGN SOIL BEARING PRESSURE IS 1000 PSF DL + LL, WITH A 1/3 INCREASE FOR SEISMIC LOADS.

SUBJECT TO THE APPROVAL OF THE CITY OR COUNTY BUILDING INSPECTOR. 5. CONTRACTOR TO PROVIDE FOR DE-WATERING OF ALL

4. THE BOTTOM ELEVATION OF ALL FOOTINGS SHALL BE

EXCAVATIONS FROM EITHER SURFACE WATER OR SEEPAGE. 6. CONTRACTOR SHALL PROTECT ALL FOUNDATION

EXCAVATIONS ON THE SITE FROM CAVING.

7. AFTER FOUNDATION EXCAVATIONS HAVE BEEN COMPLETED AND PRIOR TO PLACING REINFORCING AND FORMWORK, THE FOUNDATION BED SHALL BE INSPECTED BY THE CITY OR COUNTY BUILDING INSPECTOR. ALL LOOSE MATERIAL SHALL BE REMOVED AS DIRECTED BY THE CITY OR COUNTY BUILDING

5.4 SEISMIC DESIGN PARAMETERS

INSPECTOR.

The following estimated ground motion parameters have been established using the methods outlined in the 2013 California Building Code with reference to the acceleration contour maps provided by the U.S. Geological Survey (USGS) and the National Seismic Hazard Mapping Project (NSHMP). These ground motion parameters represent the Maximum Considered Earthquake (MCE) spectral response of seismic events experiencing 5 percent damped acceleration and having a 2 percent probability of exceedance within a 50 year period.

2013 California Building Code Seismic Parameters Parameter Seismic Design Category Site Class Short Period Spectral Acceleration, Ss 1.035 -second period spectral acceleration. S1

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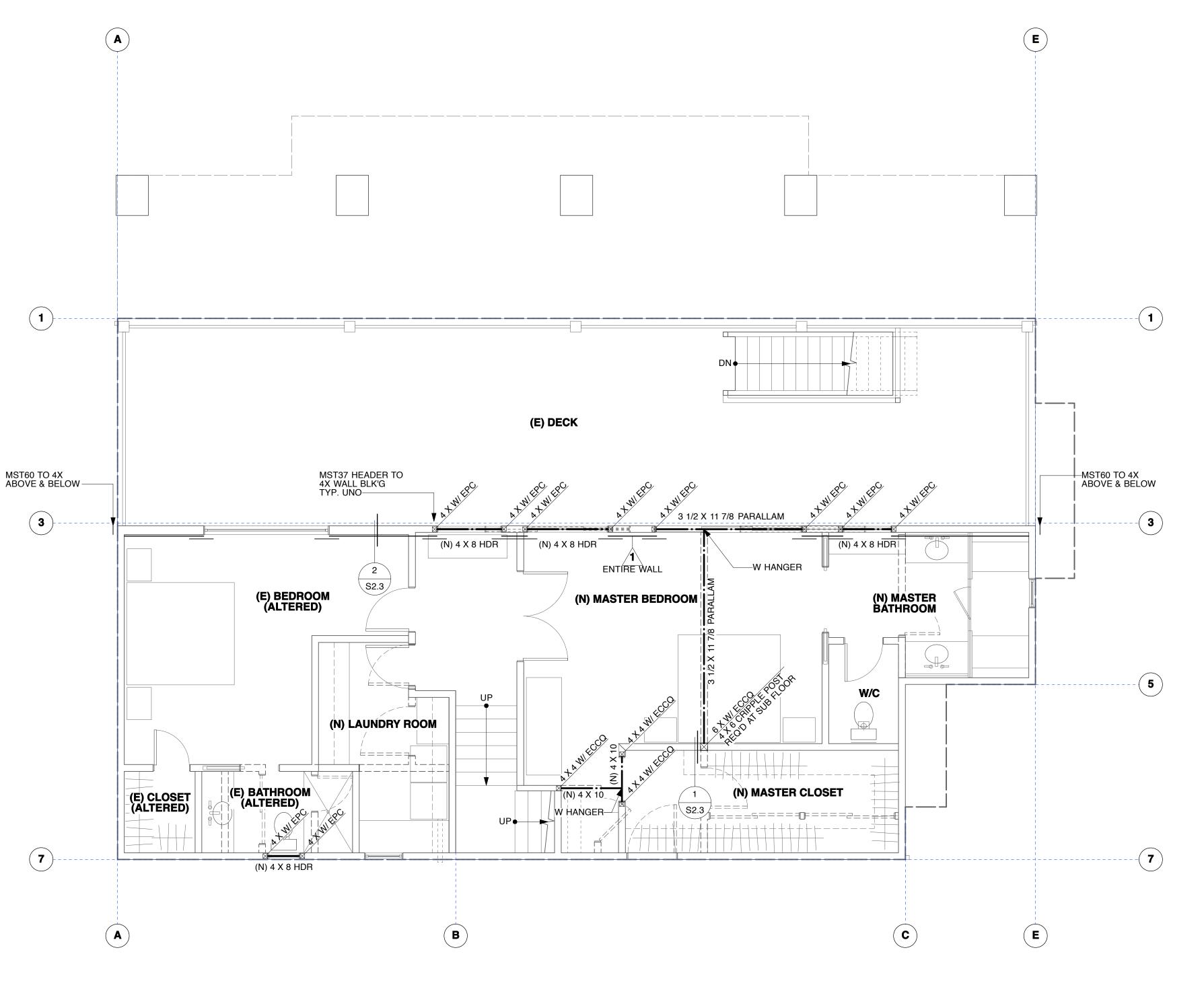
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10/08/18 SFDB RE-SUBMITTAL

DATE DESCRIPTION 09/25/18 SFDB SUBMITTAL

01/14/19 SFDB RE-SUBMITTAL 02/21/19 BLDG DEPT SUBMITTAL 04/17/19 BLDG DEPT RE-SUBMITTAL IO/31/19 SFDB REVISION #1 SUBMITTAL

GENERAL NOTES

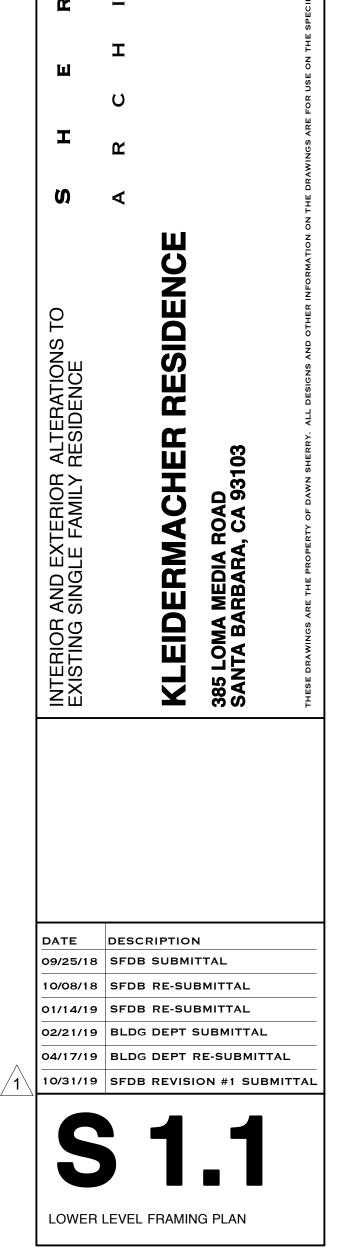


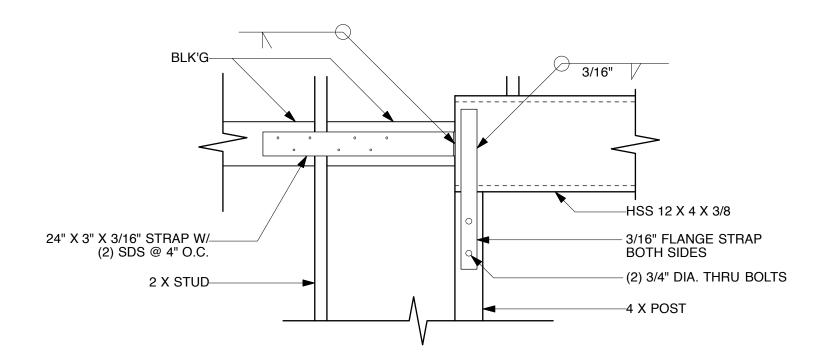


1/2" STR I PLY
10d E.N. @ 2" O.C.*
16d SILL NAIL @ 2" O.C.*

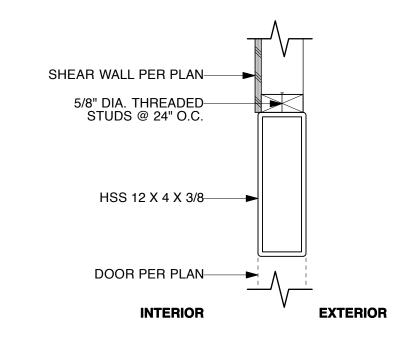
*NOTE: NAILING TO REQUIRE SPECIAL INSPECTION



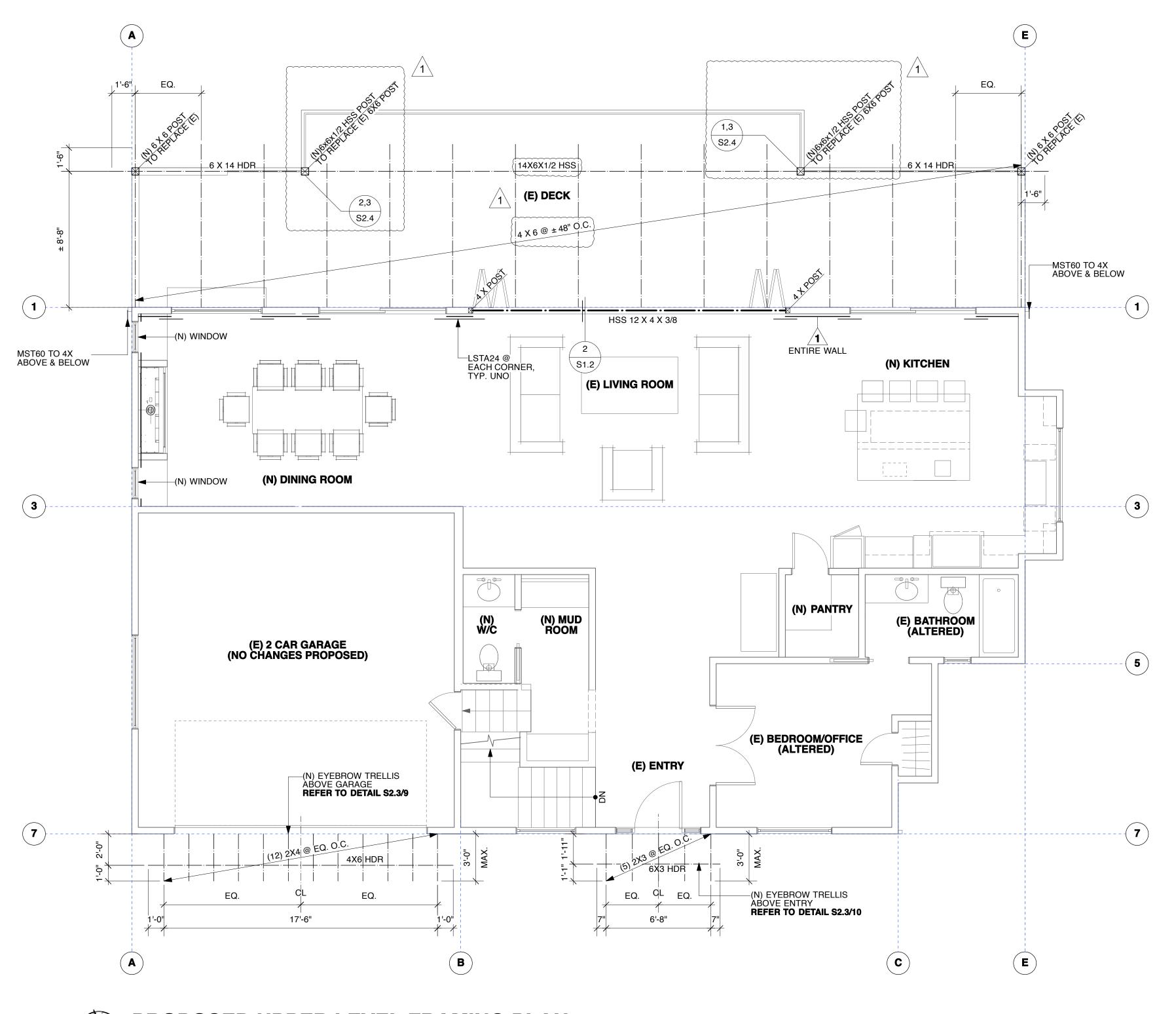




1 STEEL HEADER TO POST CONNECTION SCALE: 1" = 1'-0"



2 STEEL HEADER TO STUD CONNECTION
SCALE: 1-1/2" = 1'-0"

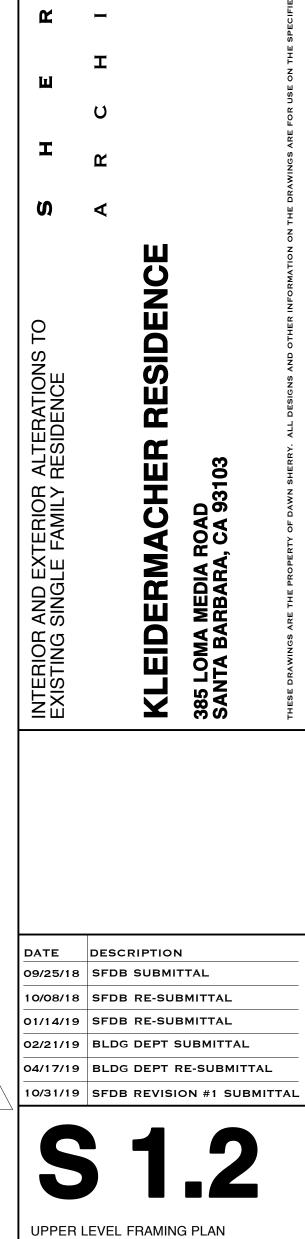


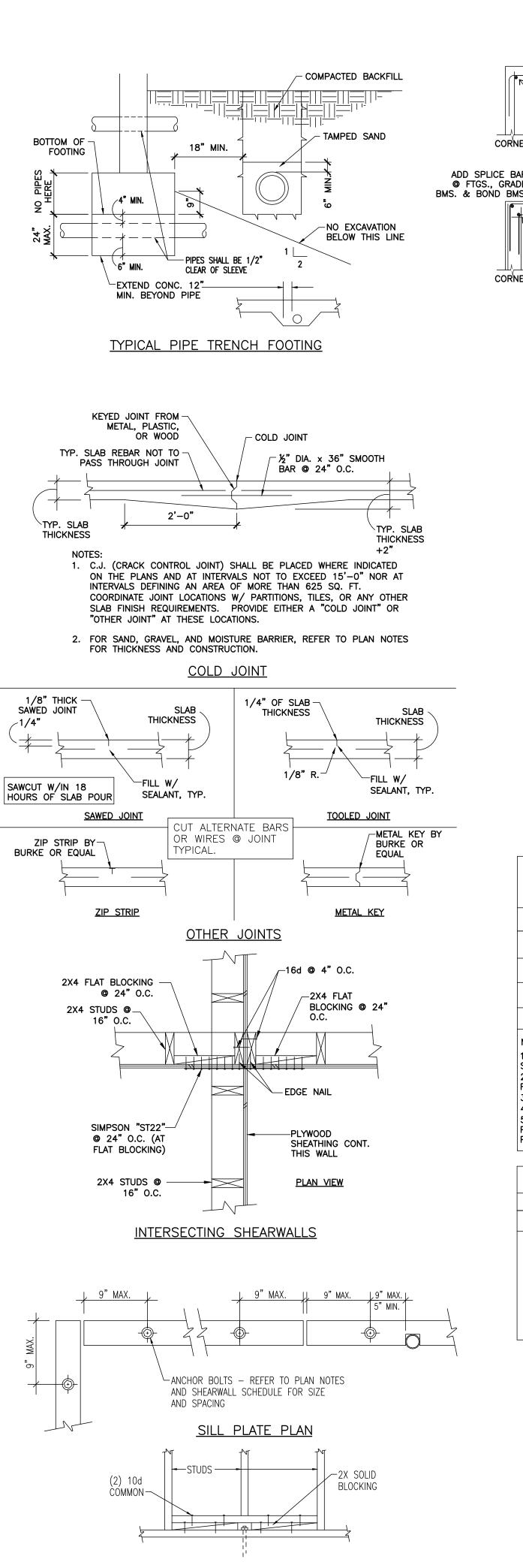


1/2" STR I PLY
10d E.N. @ 2" O.C.*
16d SILL NAIL @ 2" O.C.*

*NOTE: NAILING TO REQUIRE SPECIAL INSPECTION

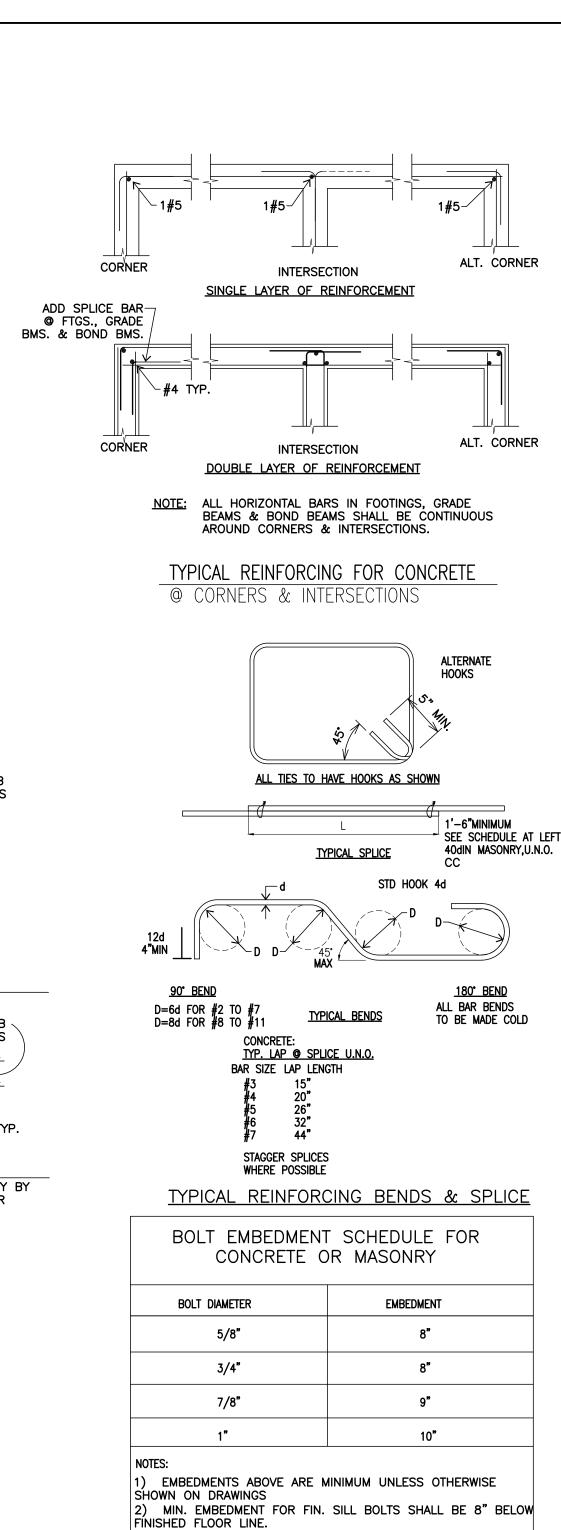




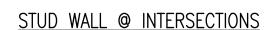


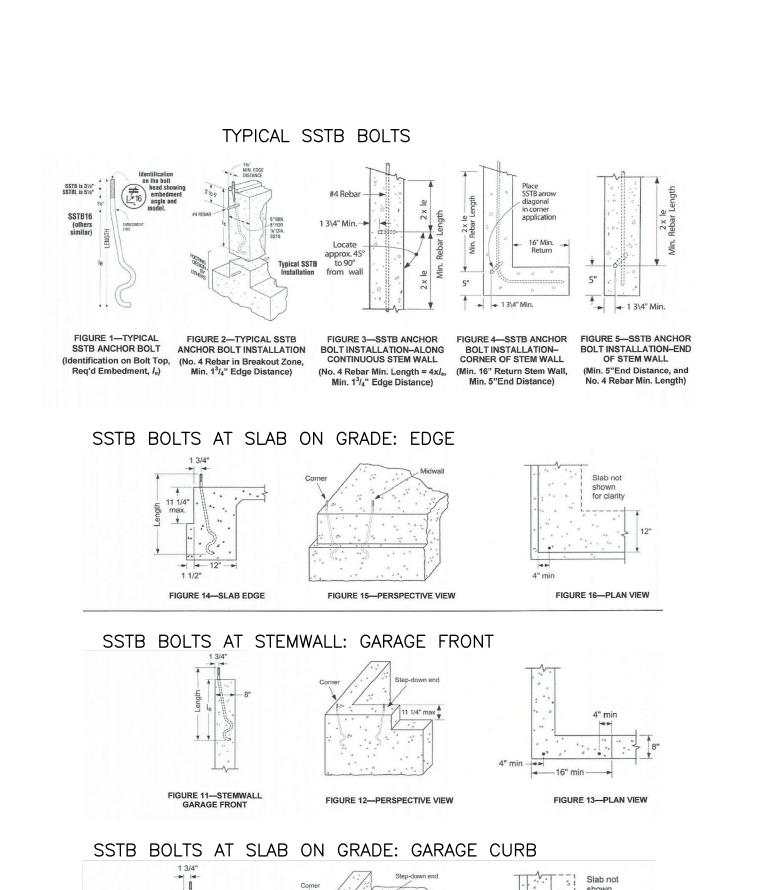
SILL PLATE ELEVATION

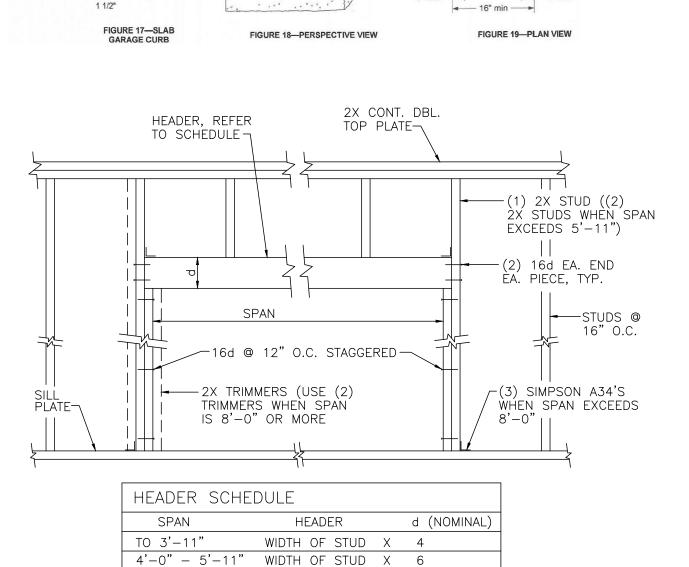
PLACING AND FRAMING AT ANCHOR BOLTS



TYPICAL REINFORG	CING BENDS & SPLICE
BOLT EMBEDMENT CONCRETE C	
BOLT DIAMETER	EMBEDMENT
5/8"	8"
3/4"	8"
7/8"	9"
1"	10"
FÍNISHED FLOOR LINE. 3) EMBEDMENT IS EXCLUSIVE 4) EMBEDMENTS ABOVE ARE II 5) CONTRACTOR RESPONSIBLE	SILL BOLTS SHALL BE 8" BELOW OF DRYPACK. NTO 1st POUR.
NAILING	SCHEDULE (1)





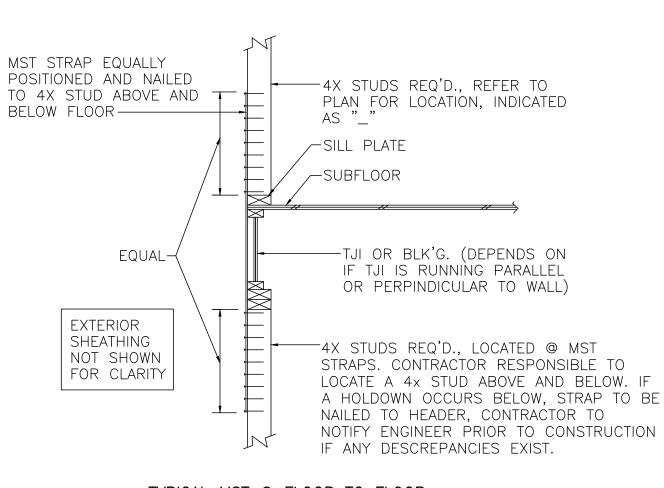


6'-0" - 7'-11" WIDTH OF STUD X 8 8'-0" - 10'-0" WIDTH OF STUD X 10

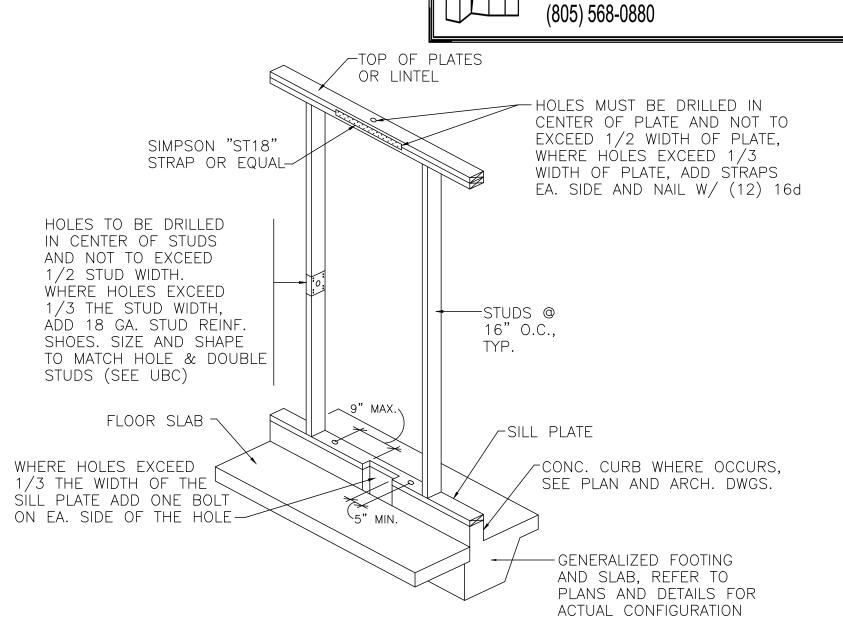
AS DIRECTED BY THE STRUCTURAL ENGINEER.

FOR SPANS GREATER THAN 10'-0", HEADER SHALL BE

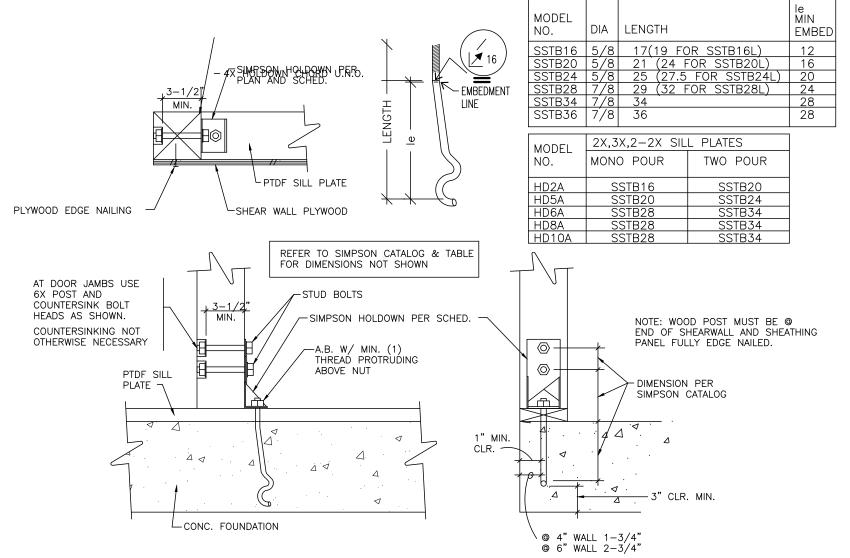
→ 12" →



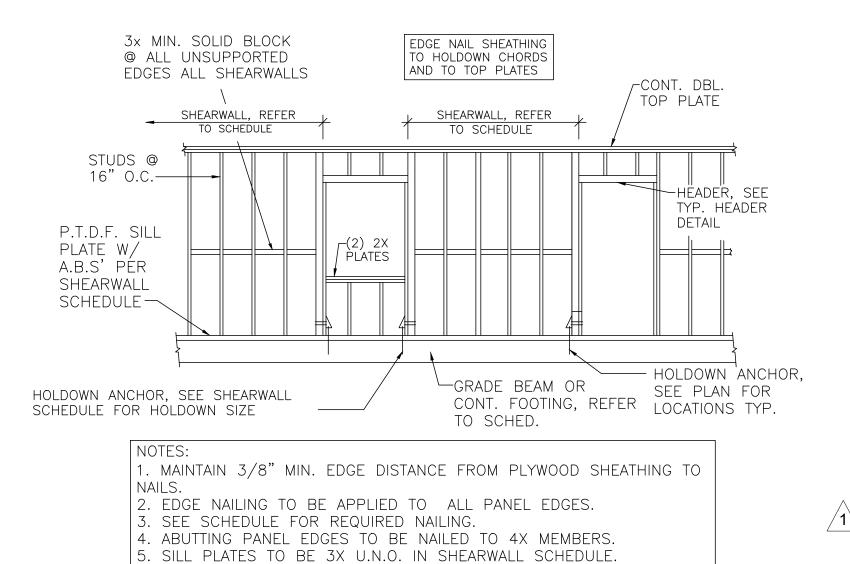
TYPICAL MST @ FLOOR TO FLOOR



TYPICAL HOLES AND NOTCHES IN STUD WALL FRAMING



TYPICAL HOLDOWN ANCHOR



6. ALL FOUNDATION WASHERS TO BE 3"x3"x.229" SQ. PLATES, U.N.O.

TYPICAL DETAILS

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DATE DESCRIPTION 09/25/18 SFDB SUBMITTAL 10/08/18 SFDB RE-SUBMITTAL 01/14/19 SFDB RE-SUBMITTAL 02/21/19 BLDG DEPT SUBMITTAL 04/17/19 BLDG DEPT RE-SUBMITTAL 10/31/19 SFDB REVISION #1 SUBMITTAL

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TABLE 1705A.3
REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

REQUIRED VER	RIFICATION AND INS	PECTION OF CO	ICRETE CONSTRUCTION	
VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD ^a	IBC REFERENCE
Inspection of reinforcing steel, including prestressing tendons, and placement.	-	X	ACI 318: 3.5, 7.1-7.7	1910.4
Inspection of reinforcing steel welding in accordance with Table 1705.2.2, Item 2b.	9_2	-	AWS D1.4 ACI 318: 3.5.2	S <u>—</u>
Inspection of anchors cast in concrete where allowable loads have been increased or where strength design is used.	-	X	ACI 318: 8.1.3, 21.2.8	1908.5, 1909.1
4. Inspection of anchors post-installed in hardened concrete members ^b .	<u> </u>	X	ACI 318: 3.8.6, 8.1.3, 21.2.8	1909.1
5. Verifying use of required design mix.	÷	X	ACI 318: Ch. 4, 5.2-5.4	1904.2, 1910.2, 1910.3
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	Х	(2000)	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1910.10
7. Inspection of concrete and shotcrete placement for proper application techniques.	X	-	ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8
8. Inspection for maintenance of specified curing temperature and techniques.	8	X	ACI 318: 5.11-5.13	1910.9
9. Inspection of prestressed concrete: a. Application of prestressing forces. b. Grouting of bonded prestressing tendons in the seismic force-resisting system.	X X	F	ACI 318: 18.20 ACI 318: 18.18.4	2 <u>——</u> 1
10. Erection of precast concrete members.	\$ 5 15 4	X	ACI 318: Ch. 16	8
11. Verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	<u></u>	Х	ACI 318: 6.2	s
12. Inspect formwork for shape, location and dimensions of the concrete member being formed.	-	X	ACI 318: 6.1.1	1
13. Inspection of adhesive anchors in horizontal and upwardly inclined positions.°	-	X	ACI 318: D.9.2.2	5—————————————————————————————————————

For SI: 1 inch = 25.4 mm.

a. Where applicable, see also Section 1705.11, Special inspections for seismic resistance.
b. Specific requirements for special inspection shall be included in the research report for the anchor issued by an approved source in accordance with ACI 355.2 or other qualification procedures. Where specific requirements are not provided, special inspection requirements shall be specified by the registered design professional and shall be approved by the building official prior to the commencement of the work.

TABLE 1705*A*.6 REQUIRED VERIFICATION AND INSPECTION OF SOILS

c. Installation of all adhesive anchors in horizontal and upwardly inclined positions shall be performed by an ACI/CRSI certified adhesive anchor installer.

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	_	X
Verify excavations are extended to proper depth and have reached proper material.		X
3. Perform classification and testing of compacted fill materials.	()	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	X	-
5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.	=	X

	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
		Roof	8
1.	Blocking between ceiling joists, rafters or trusses to top plate or other framing below	3-8d common (2 ¹ / ₂ " × 0.131"); or 3-10d box (3" × 0.128"); or 3-3" × 0.131" nails; or 3-3" 14 gage staples, ⁷ / ₁₆ " crown	Each end, toenail
	Blocking between rafters or truss not at the wall	2-8d common (2 ¹ / ₂ " × 0.131") 2-3" × 0.131" nails 2-3" 14 gage staples	Each end, toenail
	top plate, to rafter or truss	2-16 d common (3 ¹ / ₂ " × 0.162") 3-3" × 0.131" nails 3-3" 14 gage staples	End nail
	Flat blocking to truss and web filler	16d common (3 ¹ / ₂ " × 0.162") @ 6" o.c. 3" × 0.131" nails @ 6" o.c. 3" × 14 gage staples @ 6" o.c	Face nail
2.	Ceiling joists to top plate	3-8d common (2 ¹ / ₂ " × 0.131"); or 3-10d box (3" × 0.128"); or 3-3" × 0.131" nails; or 3-3" 14 gage staples, ⁷ / ₁₆ " crown	Each joist, toenail
3.	Ceiling joist not attached to parallel rafter, laps over partitions (no thrust) (see Section 2308.7.3.1, Table 2308.7.3.1)	3-16d common (3 ¹ / ₂ " × 0.162"); or 4-10d box (3" × 0.128"); or 4-3" × 0.131" nails; or 4-3" 14 gage staples, ⁷ / ₁₆ " crown	Face nail
4.	Ceiling joist attached to parallel rafter (heel joint) (see Section 2308.7.3.1, Table 2308.7.3.1)	Per Table 2308.7.3.1	Face nail
5.	Collar tie to rafter	3-10d common (3" × 0.148"); or 4-10d box (3" × 0.128"); or 4-3" × 0.131" nails; or 4-3" 14 gage staples, ⁷ / ₁₆ " crown	Face nail
6.	Rafter or roof truss to top plate (See Section 2308.7.5, Table 2308.7.5)	3-10 common (3" × 0.148"); or 3-16d box (3 ¹ / ₂ " × 0.135"); or 4-10d box (3" × 0.128"); or 4-3" × 0.131 nails; or 4-3" 14 gage staples, ⁷ / ₁₆ " crown	Toenail ^c
7	Professional Control	2-16d common $(3^1/_2" \times 0.162")$; or 3-10d box $(3" \times 0.128")$; or 3-3" $\times 0.131"$ nails; or 3-3" 14 gage staples, $7/_{16}"$ crown; or	End nail
1.	Roof rafters to ridge valley or hip rafters; or roof rafter to 2-inch ridge beam	3-10d common $(3^1/_2" \times 0.148")$; or 3-16d box $(3^1/_2" \times 0.135")$; or 4-10d box $(3" \times 0.128")$; or 4-3" $\times 0.131"$ nails; or 4-3" 14 gage staples, $\frac{7}{16}$ " crown	Toenail

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
	Wall	
	16d common $(3^1/2'' \times 0.162'')$;	24" o.c. face nail
8. Stud to stud (not at braced wall panels)	10d box (3" × 0.128"); or 3" × 0.131" nails; or 3-3" 14 gage staples, $\frac{7}{16}$ " crown	16" o.c. face nail
	16d common $(3^1/_2'' \times 0.162'')$; or	16" o.c. face nail
9. Stud to stud and abutting studs at intersecting wall	16d box $(3^1/_2'' \times 0.135'')$; or	12" o.c. face nail
corners (at braced wall panels)	3" × 0.131" nails; or 3-3" 14 gage staples, ⁷ / ₁₆ " crown	12" o.c. face nail
	16d common $(3^{1}/_{2}" \times 0.162")$; or	16" o.c. each edge, face nail
10. Built-up header (2" to 2" header)	16d box (3 ¹ / ₂ " × 0.135")	12" o.c. each edge, face nail
11. Continuous header to stud	4-8d common $(2^{1}/_{2}'' \times 0.131'')$; or 4-10d box $(3'' \times 0.128'')$	Toenail
	16d common $(3^1/_2'' \times 0.162'')$; or	16" o.c. face nail
12. Top plate to top plate	10d box (3" × 0.128"); or 3" × 0.131" nails; or 3" 14 gage staples, ⁷ / ₁₆ " crown	12" o.c. face nail
13. Top plate to top plate, at end joints	8-16d common (3 ¹ / ₂ " × 0.162"); or 12-10d box (3" × 0.128"); or 12-3" × 0.131" nails; or 12-3" 14 gage staples, ⁷ / ₁₆ " crown	Each side of end joint, face nail (minimum 24" lap splice length each side of end joint)
	16d common $(3^{1}/_{2}'' \times 0.162'')$; or	16" o.c. face nail
 Bottom plate to joist, rim joist, band joist or block- ing (not at braced wall panels) 	16d box $(3^{1}/_{2}'' \times 0.135'')$; or $3'' \times 0.131''$ nails; or $3''$ 14 gage staples, ${}^{7}/_{16}''$ crown	12" o.c. face nail
 Bottom plate to joist, rim joist, band joist or block- ing at braced wall panels 	2-16d common $(3^{1}/_{2}'' \times 0.162'')$; or 3-16d box $(3^{1}/_{2}'' \times 0.135'')$; or 4-3" × 0.131" nails; or	16" o.c. face nail
T man I	4-3" 14 gage staples, $\frac{7}{16}$ " crown	
16. Stud to top or bottom plate	4-8d common (2 ¹ / ₂ " × 0.131"); or 4-10d box (3" × 0.128"); or 4-3" × 0.131" nails; or 4-3" 14 gage staples, ⁷ / ₁₆ " crown; or	Toenail
to. Stud to top of bottom plate	2-16d common ($3^{1}/_{2}'' \times 0.162''$); or 3-10d box ($3'' \times 0.128''$); or 3-3" × 0.131" nails; or 3-3" 14 gage staples, ${}^{7}/_{16}$ " crown	End nail
17. Top or bottom plate to stud	2-16d common ($3^{1}/_{2}'' \times 0.162''$); or 3-10d box ($3'' \times 0.128''$); or 3-3" × 0.131" nails; or	End nail

3-3" 14 gage staples, ⁷/₁₆" crown

 $3-3'' \times 0.131''$ nails; or

3-3" × 0.131" nails; or

18. Top plates, laps at corners and intersections

3-3" 14 gage staples, ⁷/₁₆" crown 2-16d common $(3^{1}/_{2}'' \times 0.162'')$; or $3-10d \text{ box } (3'' \times 0.128''); \text{ or }$

Face nail

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
5.	Wall	*
19. 1" brace to each stud and plate	2-8d common (2 ¹ / ₂ " × 0.131"); or 2-10d box (3" × 0.128"); or 2-3" × 0.131" nails; or 2-3" 14 gage staples, ⁷ / ₁₆ " crown	Face nail
20 1" v 6" sheathing to each bearing	2-8d common $(2^1/_2'' \times 0.131'')$; or 2-10d box $(3'' \times 0.128'')$	Face nail
	3-8d common $(2^1/_2'' \times 0.131'')$; or 3-10d box $(3'' \times 0.128'')$	Face nail
	Floor	*
22. Joist to sill, top plate, or girder	3-8d common (2 ¹ / ₂ " × 0.131"); or floor 3-10d box (3" × 0.128"); or 3-3" × 0.131" nails; or 3-3" 14 gage staples, ⁷ / ₁₆ " crown	Toenail
23. Rim joist, band joist, or blocking to top plate, sill or other framing below	8d common $(2^1/_2'' \times 0.131'')$; or 10d box $(3'' \times 0.128'')$; or 3" $\times 0.131''$ nails; or 3" 14 gage staples, ${}^7/_{16}$ " crown	6" o.c., toenail
	2-8d common (2 ¹ / ₂ " × 0.131"); or 2-10d box (3" × 0.128")	Face nail
25. 2" subfloor to joist or girder	2-16d common $(3^1/_2'' \times 0.162'')$	Face nail
26. 2" planks (plank & beam – floor & roof)	2-16d common $(3^1/_2'' \times 0.162'')$	Each bearing, face nail
	20d common (4" × 0.192")	32" o.c., face nail at top and bot tom staggered on opposite sides
	10d box (3" × 0.128"); or 3" × 0.131" nails; or 3" 14 gage staples, ⁷ / ₁₆ " crown	24" o.c. face nail at top and bot tom staggered on opposite sides
	And: 2-20d common (4" × 0.192"); or 3-10d box (3" × 0.128"); or 3-3" × 0.131" nails; or 3-3" 14 gage staples, ⁷ / ₁₆ " crown	Ends and at each splice, face na
28. Ledger strip supporting joists or rafters	3-16d common (3 ¹ / ₂ " × 0.162"); or 4-10d box (3" × 0.128"); or 4-3" × 0.131" nails; or 4-3" 14 gage staples, ⁷ / ₁₆ " crown	Each joist or rafter, face nail
29. Joist to band joist or rim joist	3-16d common (3 ¹ / ₂ " × 0.162"); or 4-10d box (3" × 0.128"); or 4-3" × 0.131" nails; or 4-3" 14 gage staples, ⁷ / ₁₆ " crown	End nail
30. Bridging or blocking to joist, rafter or truss	2-8d common $(2^1/_2'' \times 0.131'')$; or 2-10d box $(3'' \times 0.128'')$; or 2-3" $\times 0.131''$ nails; or 2-3" 14 gage staples, $\frac{7}{16}$ " crown	Each end, toenail

TABLE 2304.10.1—continued

FASTENING SCHEDULE

DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER		IG AND LOCATION
Wood structural panels (WSP), subfloor, roo	of and interior wall sheathing to framing and particleboa	rd wall sheath	ing to framing ^a
		Edges (inches)	Intermediate support (inches)
	6d common or deformed (2" × 0.113") (subfloor and wall)	6	12
	8d box or deformed $(2^{1}/_{2}" \times 0.113")$ (roof)	6	12
31. ${}^{3}I_{8}'' - {}^{1}I_{2}''$	$2^{3}/_{8}'' \times 0.113''$ nail (subfloor and wall)	6	12
$I_8 - I_2$	1 ³ / ₄ " 16 gage staple, ⁷ / ₁₆ " crown (subfloor and wall)	4	8
	2 ³ / ₈ " × 0.113" nail (roof)	4	8
	1 ³ / ₄ " 16 gage staple, ⁷ / ₁₆ " crown (roof)	3	6
32. ¹⁹ / ₃₂ " – ³ / ₄ "	8d common (2 ¹ / ₂ " × 0.131"); or 6d deformed (2" × 0.113")	6	12
$I_{32} - I_4$	$2^{3}l_{8}'' \times 0.113''$ nail; or 2" 16 gage staple, ${}^{7}l_{16}''$ crown	4	8
33. ⁷ / ₈ " – 1 ¹ / ₄ "	10d common (3" × 0.148"); or 8d deformed (2 ¹ / ₂ " × 0.131")	6	12
	Other exterior wall sheathing		
34. 1/2" fiberboard sheathing ^b	$1^{1}l_{2}^{"}$ galvanized roofing nail $({}^{7}l_{16}^{"}$ head diameter); or $1^{1}l_{4}^{"}$ 16 gage staple with ${}^{7}l_{16}^{"}$ or 1" crown	3	6
35. ²⁵ / ₃₂ " fiberboard sheathing ^b	1 ³ / ₄ " galvanized roofing nail (⁷ / ₁₆ " diameter head); or 1 ¹ / ₂ " 16 gage staple with ⁷ / ₁₆ " or 1" crown	3	6
Wood structura	l panels, combination subfloor underlayment to framing	I.	
36. 3/ ₄ " and less	8d common (2 ¹ / ₂ " × 0.131"); or 6d deformed (2" × 0.113")	6	12
37. ⁷ / ₈ " – 1"	8d common (2 ¹ / ₂ " × 0.131"); or 8d deformed (2 ¹ / ₂ " × 0.131")	6	12
38. 1 ¹ / ₈ " – 1 ¹ / ₄ "	10d common (3" × 0.148"); or 8d deformed ($2^{1}/_{2}$ " × 0.131")	6	12
	Panel siding to framing		0.0
39. ¹ / ₂ " or less	6d corrosion-resistant siding $(1^7 l_8'' \times 0.106'')$; or 6d corrosion-resistant casing $(2'' \times 0.099'')$	6	12
40. ⁵ / ₈ "	8d corrosion-resistant siding (2 ³ / ₈ " × 0.128"); or 8d corrosion-resistant casing (2 ¹ / ₂ " × 0.113")	6	12
	(continued)		

TABLE 2304.10.1—continued

	FASTENING SCHEDULE		
DESCRIPTION OF BUILDING ELE	MENTS NUMBER AND TYPE OF FASTENER	SPACI	NG AND LOCATION
Wood structural panels (WSP),	subfloor, roof and interior wall sheathing to framing and particle	board wall sheath	ning to framing ^a
		Edges (inches)	Intermediate supports (inches)
	Interior paneling		0.5
41. 1/4"	4d casing $(1^1/_2'' \times 0.080'')$; or 4d finish $(1^1/_2'' \times 0.072'')$	6	12
42. 3/8"	6d casing (2" × 0.099"); or 6d finish (Panel supports at 24 inches)	6	12

For SI: 1 inch = 25.4 mm.

a. Nails spaced at 6 inches at intermediate supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shear walls, refer to Section 2305. Nails for wall sheathing are permitted to be common, box or casing.

b. Spacing shall be 6 inches on center on the edges and 12 inches on center at intermediate supports for nonstructural applications. Panel supports at 16 inches (20 inches if strength axis in the long direction of the panel, unless otherwise marked).
c. Where a rafter is fastened to an adjacent parallel ceiling joist in accordance with this schedule and the ceiling joist is fastened to the top plate in accordance with this schedule, the number of toenails in the rafter shall be permitted to be reduced by one nail.



INTERIOR AND EXTERIOR ALTERATIONS TO EXISTING SINGLE FAMILY RESIDENCE	ທ	I	Ш	Œ	œ	>		ಹ	∢	Ø	Ø	0	U
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DATE DESCRIPTION 09/25/18 SFDB SUBMITTAL

TYPICAL DETAILS

